

PONDERAY COMMERCIAL STORAGE

BONNER MALL WAY

PONDERAY, ID 83852



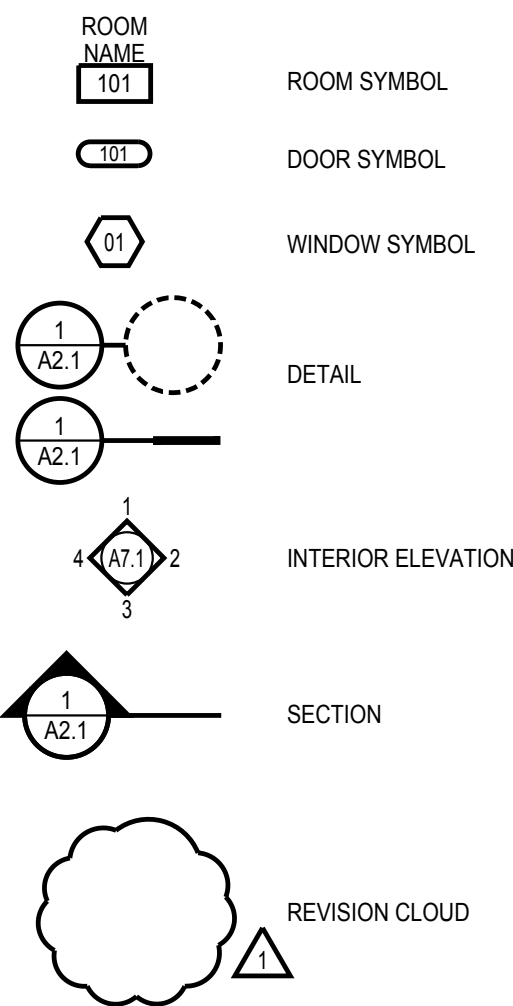
926 Peachtree Dr.
Moscow, ID 83843
208.310.0289

ABBREVIATIONS

ALSO SEE INDIVIDUAL SHEETS FOR OTHER ABBREVIATIONS NOT LISTED HERE

& L @ ⊘ #	AND ANGLE AT CENTERLINE DIAMETER OR ROUND POUND OR NUMBER
A.C.T. ACOUST. ADJ. APPROX. ARCH. A.F.F.	ACOUSTICAL CEILING TILE ACOUSTICAL ADJUSTABLE APPROXIMATE ARCHITECTURAL ABOVE FINISH FLOOR
BLDG. BLK. BLKG. BM. BOT.	BUILDING BLOCK BLOCKING BEAM BOTTOM
CAB. CER. C.F.C.I.	CABINET CERAMIC CONT. FURNISHED, CONT. INSTALLED
CLG. CLR. COL. CONC. CONN. CONST. CONT. C.T. CTSK. CTR. CPT.	CEILING CLEAR COLUMN CONCRETE CONNECTION CONSTRUCTION CONTINUOUS CERAMIC TILE COUNTERSUNK CENTER CARPET
DBL. DEPT. DTL. DIA. DIM. DN. DR. DWG. EA. ELEV. ELECT. EQ. EXIST. EXT.	DOUBLE DEPARTMENT DETAIL DIAMETER DIMENSION DOWN DOOR DRAWING EACH ELEVATION ELECTRICAL EQUAL EXISTING EXTERIOR
F.E. F.E.C. F.F. FIN. F.I.O. FLR. FLUOR. F.O.C. F.O.F. F.O.I.C.	FIRE EXTINGUISHER FIRE EXTINGUISHER CABINET FACTORY FINISH FINISH FURNISHED AND INSTALLED BY OWNER FLOOR FLUORESCENT FACE OF CONCRETE FACE OF FINISH FURNISHED BY OWNER, INSTALLED BY CONTRACTOR
F.O.S. FURR. GA. GALV. G.W.B.	FACE OF STUD FURRING GAUGE GALVANIZED GYPSUM WALL BOARD
H.C. HDR. HDWD. H.M. HORIZ. HR. HT. INCAN. ID. INSUL. INT. JT.	HOLLOW CORE HEADER HARDWOOD HOLLOW METAL HORIZONTAL HOUR HEIGHT HANDRAIL INCANDESCENT INSIDE DIAMETER (DIM.) INSULATION INTERIOR JOINT
KIT.	KITCHEN
LAM. LAV.	LAMINATE LAVATORY
MAX. MECH. MTL. MFGR. MIN. MISC. MTD. MUL.	MAXIMUM MECHANICAL METAL MANUFACTURER MINIMUM MISCELLANEOUS MOUNTED MULLION
N.I.C. NO. NOM. N.T.S.	NOT IN CONTRACT NUMBER NOMINAL NOT TO SCALE
O.F.C.I.	OWNER FURNISHED, CONT. INSTALLED
OFF. O.C. O.D.	OFFICE ON CENTER OUTSIDE DIAMETER
PTBD. P.LAM. PLY. PR. PT. PTN. PRESERV. P.	PARTICLE BOARD PLASTIC LAMINATE PLYWOOD PAIR POINT PARTITION PRESERVATIVE PAINT
Q.T.	QUARRY TILE
R. RAD. REF. REINF. REQ'D RESIL. RM. R.O. RT. R.B. R.S.	RISER RADIUS REFRIGERATOR REINFORCED REQUIRED RESILIENT ROOM ROUGH OPENING RUBBER TILE RUBBER BASE REDWOOD SIDING
S.C. SECT. SHT. SIM. SPEC. SQ. S.S. STD. STL. STOR. STRUCT. SUSP. SYMM. S.V.	SOLID CORE SECTION SHEET SIMILAR SPECIFICATION SQUARE STAINLESS STEEL STANDARD STEEL STORAGE STRUCTURAL SUSPENDED SYMMETRICAL SHEET VINYL
T. TEL. THK. T.V. TYP. T.L.	TREAD TELEPHONE THICK TELEVISION TYPICAL TRUE LENGTH
U.N.O.	UNLESS NOTED OTHERWISE
V.C.T. VERT. VEST. V.T.	VINYL COMPOSITION TILE VERTICAL VESTIBULE VINYL TILE
W/ W.COV. WD. W/O WP. WT. W.R.	WITH WALLCOVERING WOOD WITHOUT WATERPROOF WEIGHT WATER RESISTANT

DRAWING SYMBOLS



PROJECT TEAM

OWNER/BUILDER

REALM PARTNERS
215 CEDAR STREET
SANDPOINT, IDAHO 83864
P: 208-255-6650
E: TEAGUE.REALM@GMAIL.COM
CONTACT: TEAGUE MULLEN

ARCHITECT OF RECORD

B.A.D. STUDIO, PC
502 N. 7TH STREET
COEUR D'ALENE, ID 83814
P: 208-310-0289
E: JB@BADSTUDIO.US
CONTACT: JIM BOUDREAU

BUILDING INFORMATION

1. NAME OF PROJECT:	PONDERAY COMMERCIAL STORAGE
2. STREET ADDRESS:	BONNER MALL WAY PONDERAY, ID 83833
3. ARCHITECT:	B.A.D. STUDIO, PC 502 NORTH 7TH ST. COEUR D'ALENE, ID 83814 PH: 208-310-0289
4. CONTACT PERSON:	JIM BOUDREAU
5. OCCUPANCY GROUP:	S-1
6. BUILDING CONST. TYPE:	V-8
7. BUILDING AREA:	6,600 S.F.

GENERAL NOTES

- THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR ALL PERMITS AND COORDINATION OF REQUIRED INSPECTIONS.
- CONTRACTOR SHALL VERIFY AND BE RESPONSIBLE FOR ALL DIMENSIONS AND CONDITIONS ON THE JOB AND SHALL NOTIFY THE OWNER / DESIGNER OF ANY DISCREPANCIES BEFORE COMMENCING ANY WORK.
- ON-SITE SAFETY IS THE RESPONSIBILITY OF THE CONTRACTOR.
- FIELD VERIFY EXTENT OF WORK, QUANTITY OF MATERIALS REQUIRED, AND EXISTING CONDITIONS IMPACTING THE WORK SHOWN.
- THE CONTRACTOR IS RESPONSIBLE FOR THE VERIFICATION OF AND PROVIDING ADEQUATE BEARING, CONNECTIONS, ANCHORS, AND/OR NAILING OF ALL STRUCTURAL COMPONENTS.
- ALL SURFACES ADJACENT TO THE BUILDING PERIMETER ARE TO SLOPE AND DRAIN AWAY FROM THE BUILDING.
- PROVIDE EROSION CONTROL MEASURES DURING CONSTRUCTION AS REQUIRED BY THE GOVERNING AUTHORITY.
- INSTALL SAFETY GLAZING AT ALL HAZARDOUS LOCATIONS AS DEFINED BY THE CURRENT EDITION OF THE INTERNATIONAL RESIDENTIAL CODE BOTH SHEETS OF GLASS TO BE TEMPERED WHERE REQUIRED.
- HVAC, PLUMBING, & ELECTRICAL SYSTEMS: UNLESS OTHERWISE INDICATED OR SHOWN, THE H.V.A.C., PLUMBING, & ELECTRICAL MODIFICATIONS SHALL BE BIDDER DESIGNED AND CONFORM TO THE REQUIREMENTS OF THE CURRENTLY ADOPTED EDITION OF THE INTERNATIONAL BUILDING CODE, INTERNATIONAL FIRE CODE, INTERNATIONAL ELECTRICAL CODE, INTERNATIONAL MECHANICAL & PLUMBING CODE, N.E.C., N.F.B.U AND AS REQUIRED BY AND IN CONFORMANCE WITH THE OTHER REQUIREMENTS OF THE LOCAL BUILDING AUTHORITY. IN CASE OF DISCREPANCY WITH THE CONTRACT DOCUMENTS, THE GOVERNING CODES SHALL PREVAIL.
- SHOP DRAWINGS / SUBMITTALS: IT SHALL BE THE BIDDERS RESPONSIBILITY TO PROVIDE COMPLETE CALCULATIONS, RISER DIAGRAMS, DRAWINGS, DETAILS, EQUIPMENT/FIXTURE INFORMATION, AND OTHER INFORMATION AS REQUIRED AND REQUESTED BY THE GOVERNING BUILDING AUTHORITY AS NECESSARY TO OBTAIN APPROVAL. IT IS THE RESPONSIBILITY OF THE BIDDER TO CONFIRM SUCH REQUIREMENTS WITH THE GOVERNING BUILDING AUTHORITY.

GENERAL CONSTRUCTION NOTES

- CONTRACTOR:
A. VERIFY ALL CABINETS / COUNTER / FLOORING MATERIALS / FINISHES / LAYOUTS WITH OWNER PRIOR TO FABRICATION / INSTALLATION. PROVIDE CABINET SHOP DRAWINGS FOR REVIEW.
B. LAYOUT ALL WALL DIMENSIONS AND FIELD VERIFY DIMENSIONS PRIOR TO START OF CONSTRUCTION.
C. FIELD VERIFY ACCURACY AND STACK UP OF CONSTRUCTION DIMENSIONS WITH INTERFACES TO VENDOR PRODUCTS BEFORE PROCEEDING TO SUBSEQUENT PHASES OF CONSTRUCTION.
- DIMENSIONS: ALL DIMENSIONS ARE TAKEN FROM THE FACE OF STUD WALLS AND / OR THE OUTSIDE FACE OF FOUNDATION WALL UNLESS OTHERWISE NOTED. ALL WINDOWS OR GROUPS OF WINDOWS ARE DIMENSIONED TO CENTER.
- HEADERS: ALL HEADERS (2) 2x10 DOUGLAS FIR #2, UNLESS NOTED OTHERWISE.
- AIR GAPS: ALL AIR GAPS BETWEEN FRAMING CONNECTIONS, WINDOWS AND DOORS EXPOSED TO THE EXTERIOR SHALL BE FILLER WITH AN EXPANDING POLY INSULATING FOAM SEALER.
- HANDRAILS: PROVIDE HANDRAILS ON ANY STAIRWAY WITH MORE THAN (3) RISERS - ALL HANDRAILS TO BE MOUNTED 36" ABOVE STAIR TREADS AND MUST COMPLY WITH GRASP-ABILITY REQUIREMENTS I.B.C. 1012.3.

GOVERNING CODES

ALL CONSTRUCTION SHALL COMPLY WITH THE FOLLOWING CODES, AMENDMENTS AND ORDINANCES AS REQUIRED BY BONNER COUNTY AND THE STATE OF IDAHO, AND ALL OTHER RECOGNIZED JURISDICTIONS HAVING AUTHORITY OVER THE PROJECT.

SEE SHEET A100 FOR CODES AND CODE ANALYSIS.

GENERAL STRUCTURAL NOTES

ALL CONSTRUCTION PER 2018 EDITION OF THE INTERNATIONAL BUILDING CODE.

CONTRACTOR TO COORDINATE AND VERIFY DIMENSIONS, ELEVATIONS AND DETAILS WITHIN ALL DRAWINGS. IF OMISSIONS OR DISCREPANCIES ARE NOTED, CONTRACTOR TO CONTACT DESIGNER AND / OR STRUCTURAL ENGINEER FOR CLARIFICATION BEFORE BID AND / OR CONSTRUCTION.

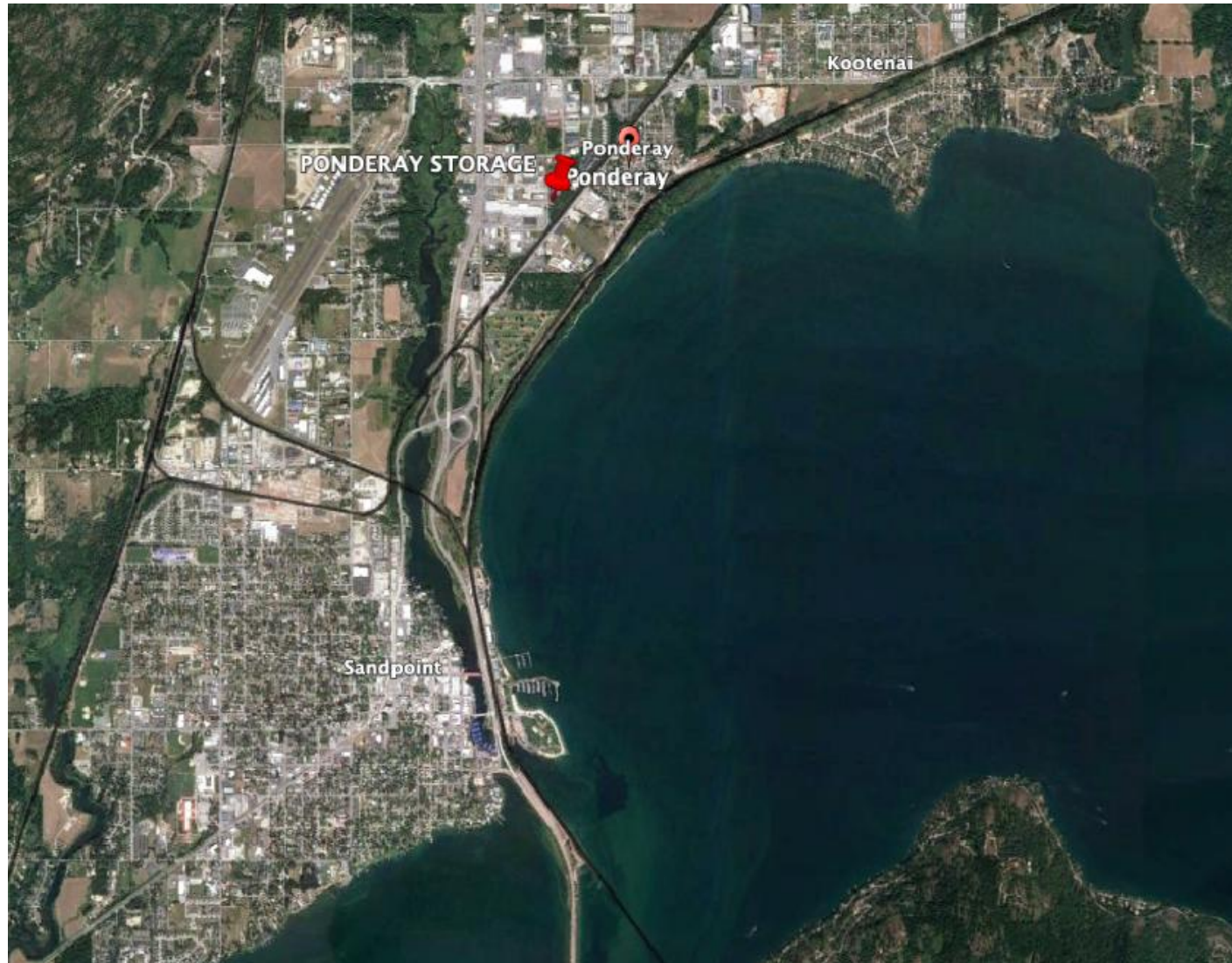
SHEET LIST

A000 COVER SHEET/NOTES/VICINITY MAP

ARCHITECTURAL
A100 NOTES/LEGENDS
A101 OVERALL FLOOR PLAN
A102 REFLECTED CEILING PLAN
A103 ROOF PLAN
A201 ELEVATIONS
A202 ELEVATIONS
A301 BUILDING SECTIONS
A302 WALL SECTIONS

STRUCTURAL

S100 LEGENDS / NOTES
S101 FOUNDATION PLAN
S102 MEZZANINE FRAMING PLAN
S103 ROOF FRAMING PLAN
S201 DETAILS
S202 DETAILS



1 VICINITY MAP
SCALE: N.T.S.



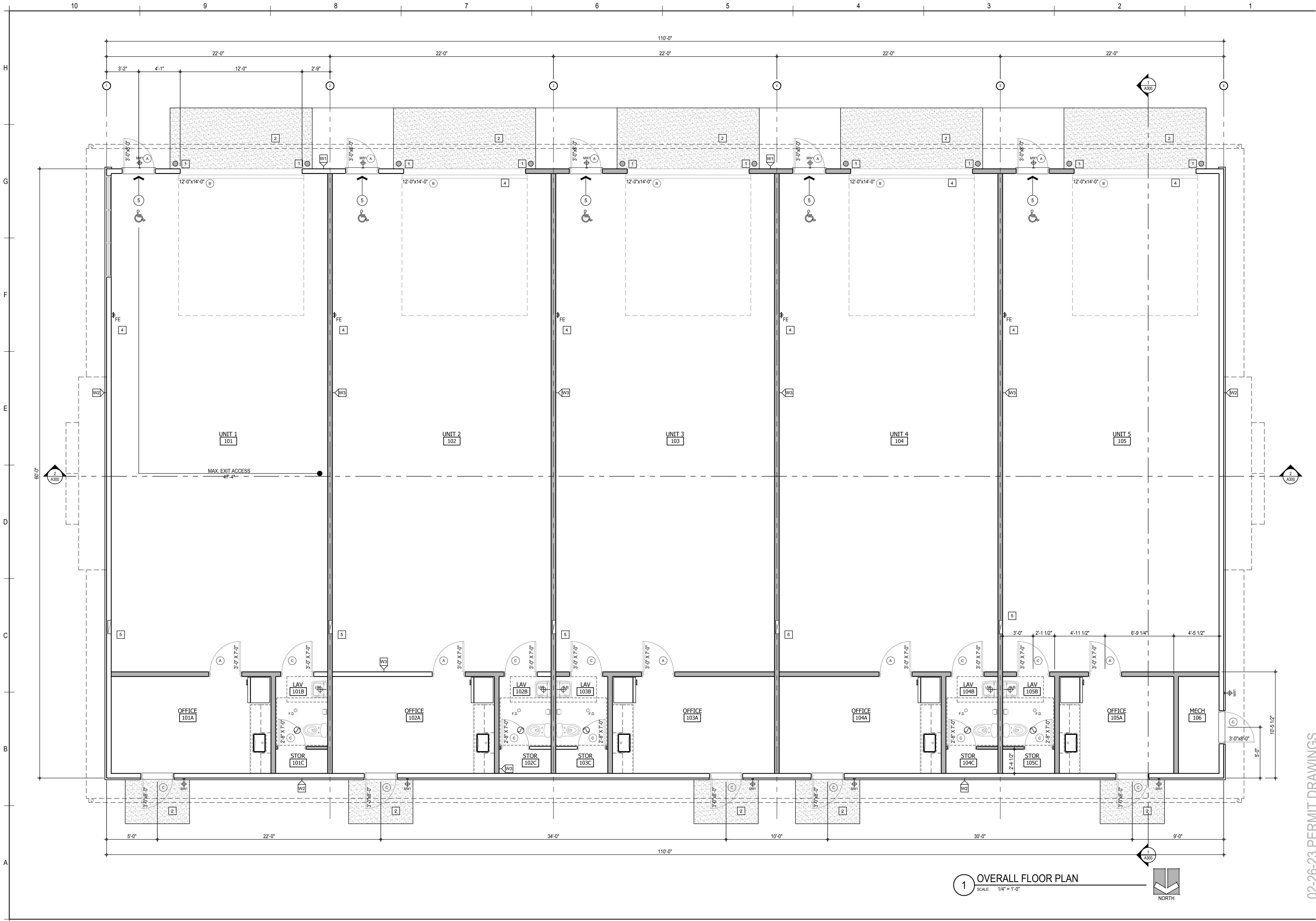
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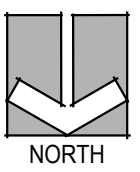
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A000

PONDERAY COMMERCIAL STORAGE
BONNER MALL WAY
PONDERAY, ID 83852



1 OVERALL FLOOR PLAN
SCALE: 1/4" = 1'-0"



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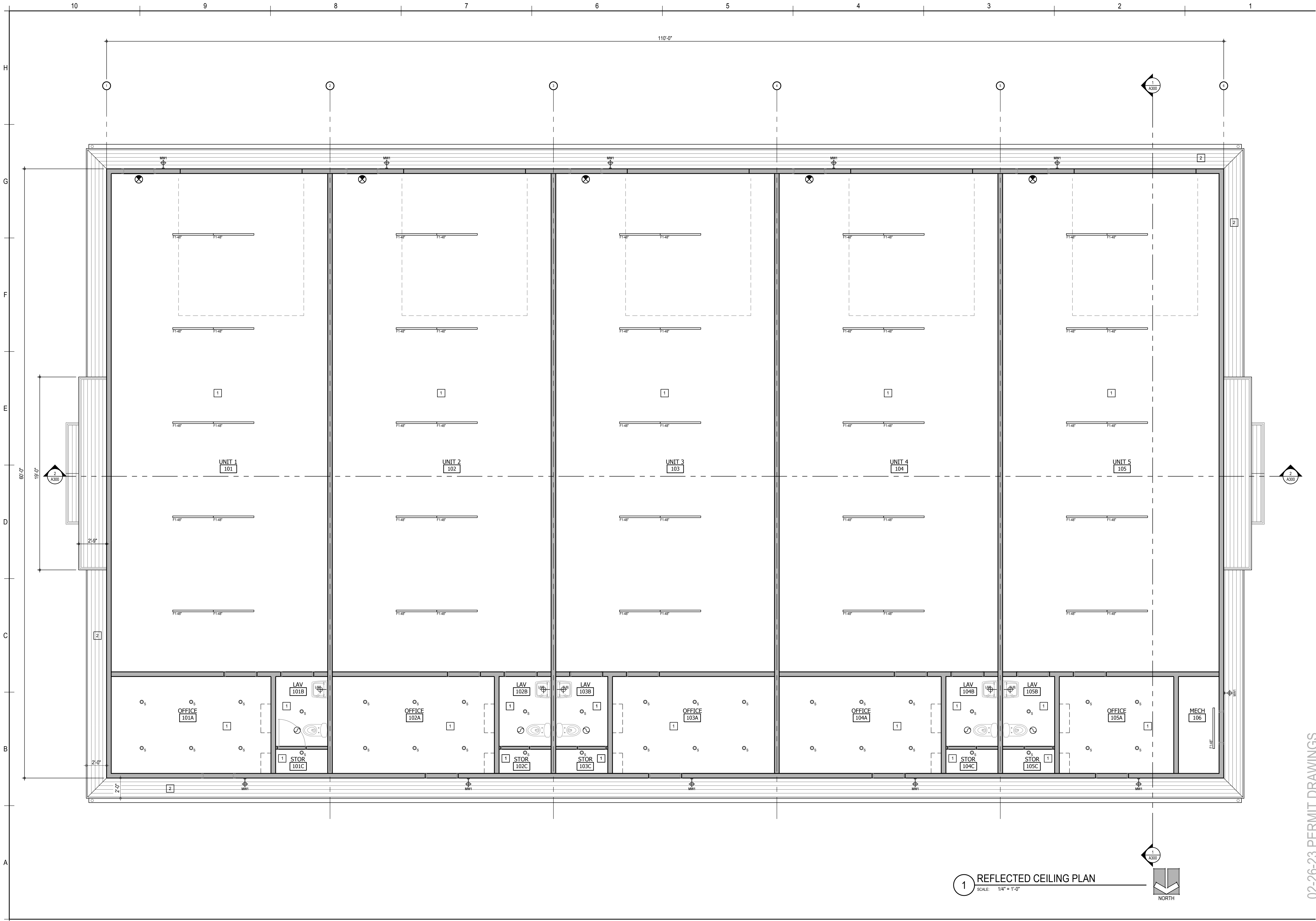
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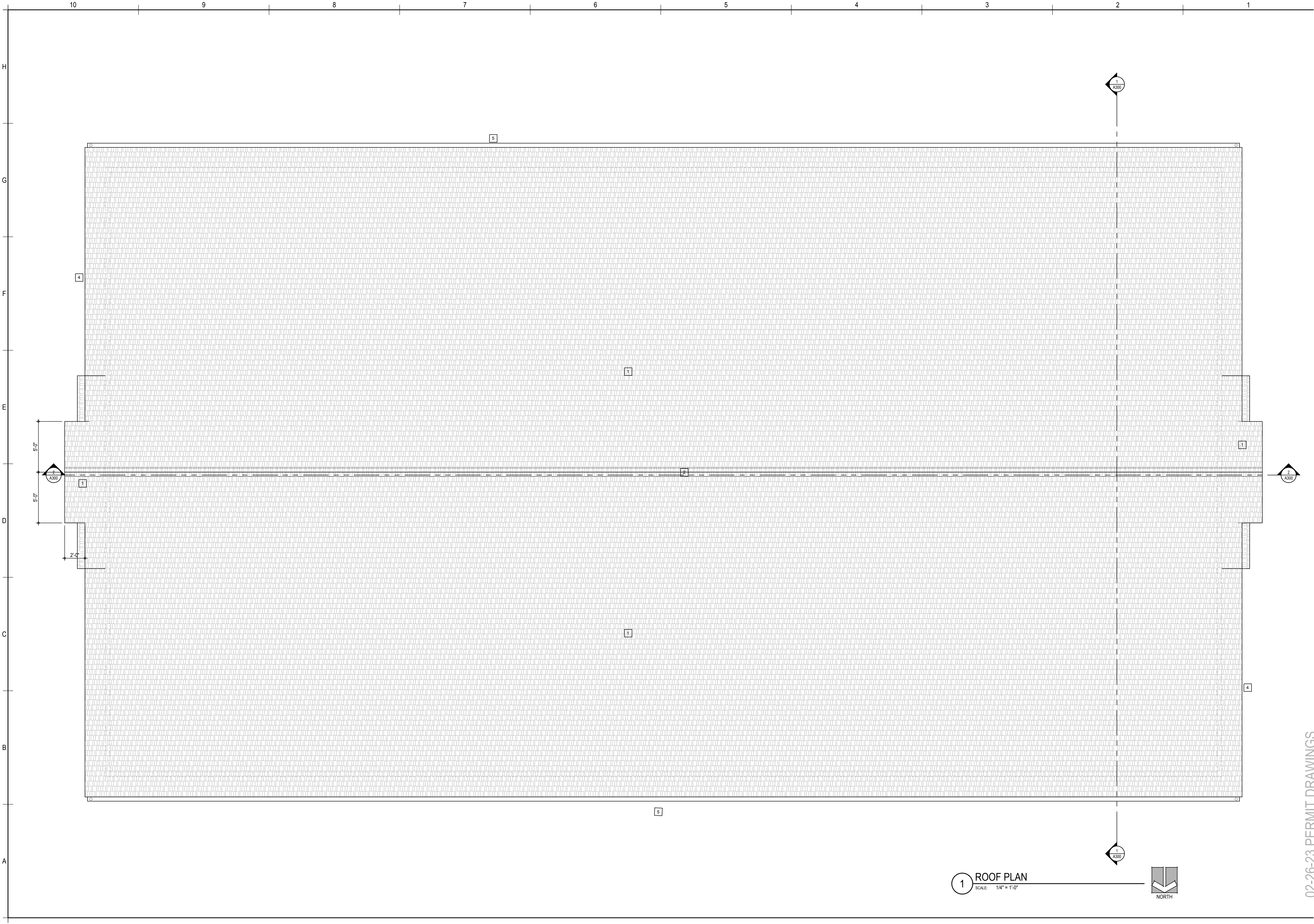
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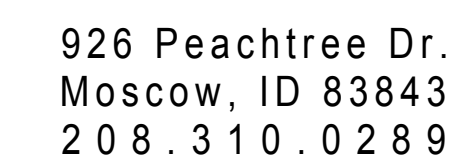

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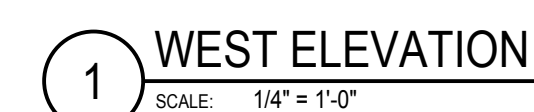
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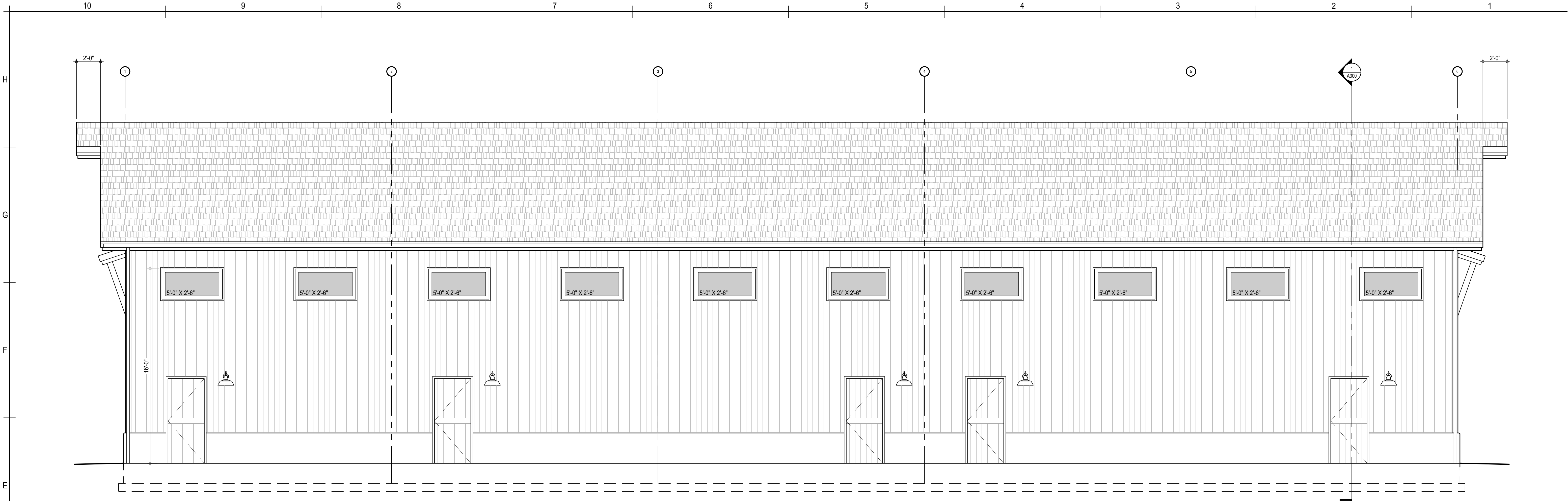
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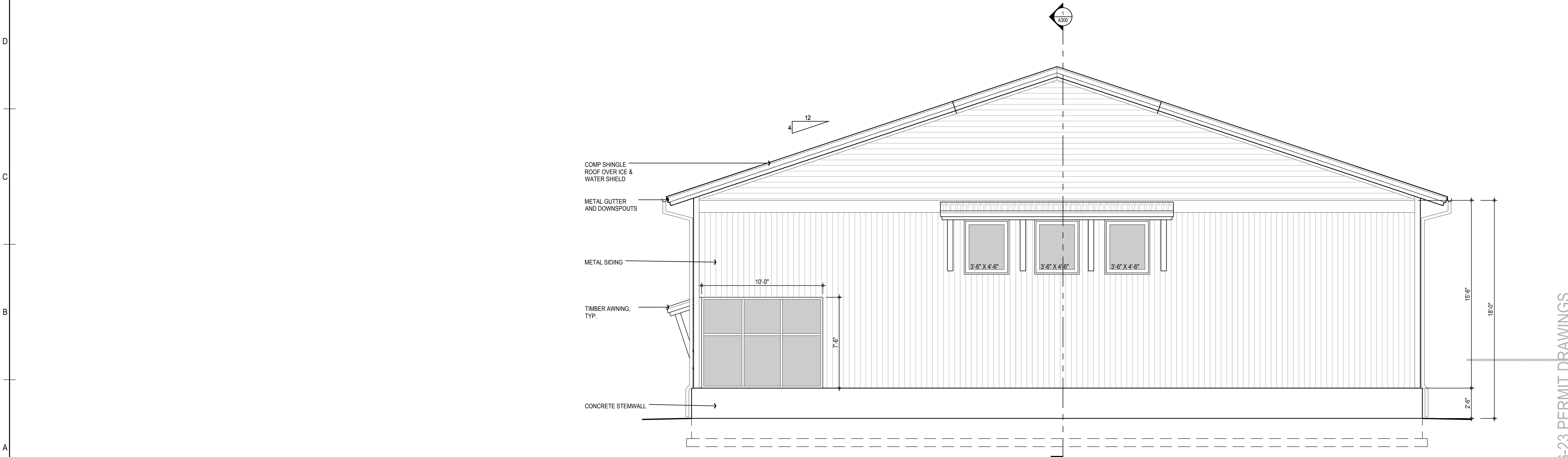
A201

002-26-23 PERMIT DRAWINGS





2 NORTH ELEVATION
SCALE: 1/4" = 1'-0"



1 EAST ELEVATION
SCALE: 1/4" = 1'-0"



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PCS BLDG 1

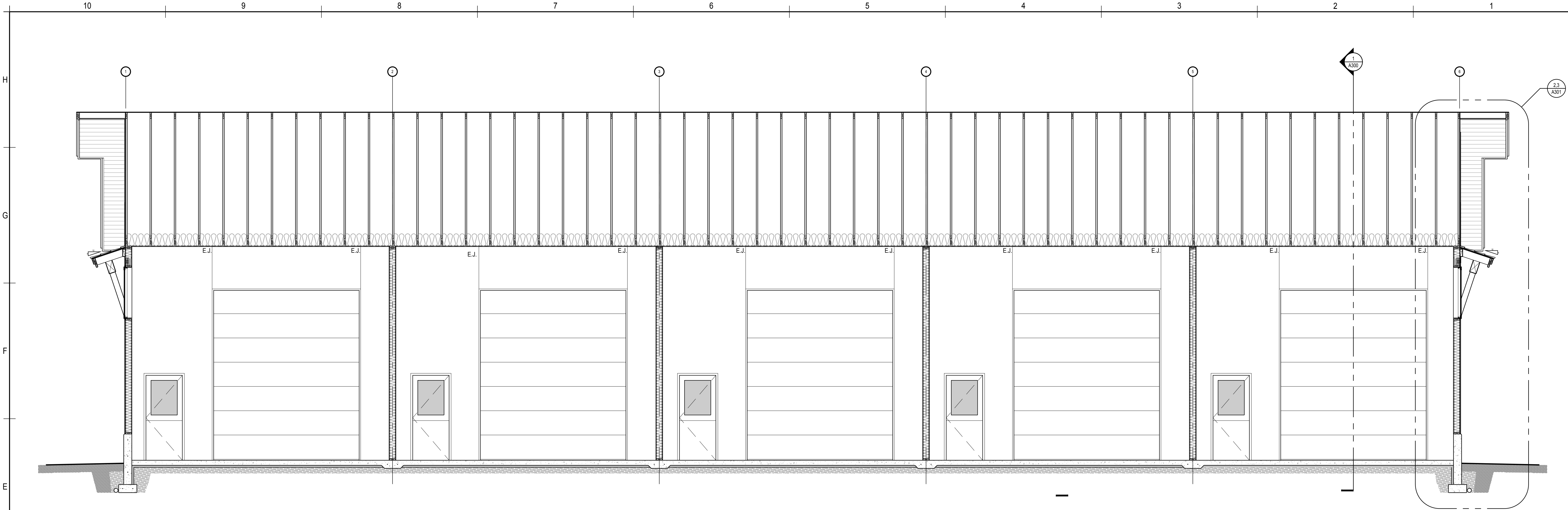
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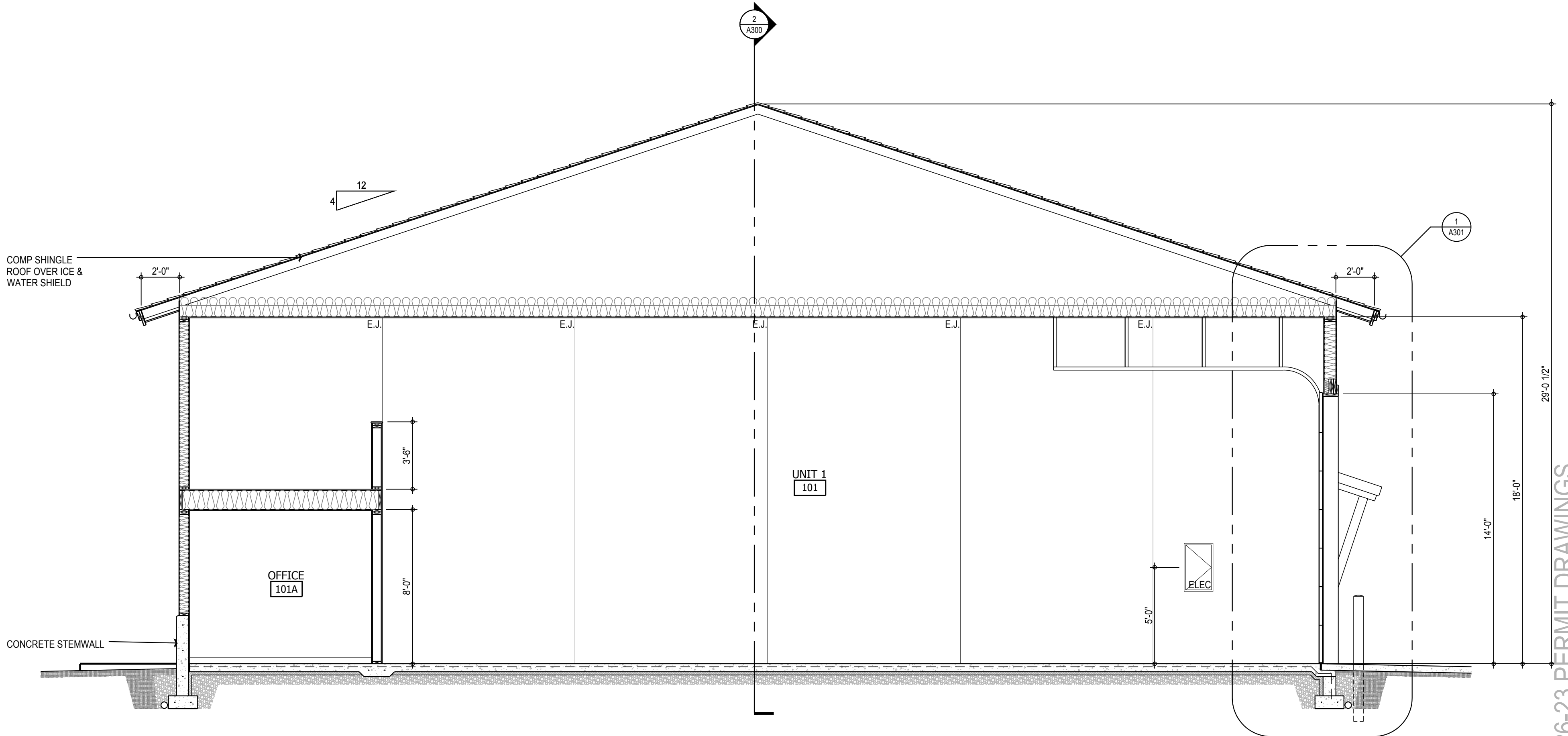
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2 BUILDING SECTION
SCALE: 1/4" = 1'-0"



1 BUILDING SECTION
SCALE: 1/4" = 1'-0"



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A301

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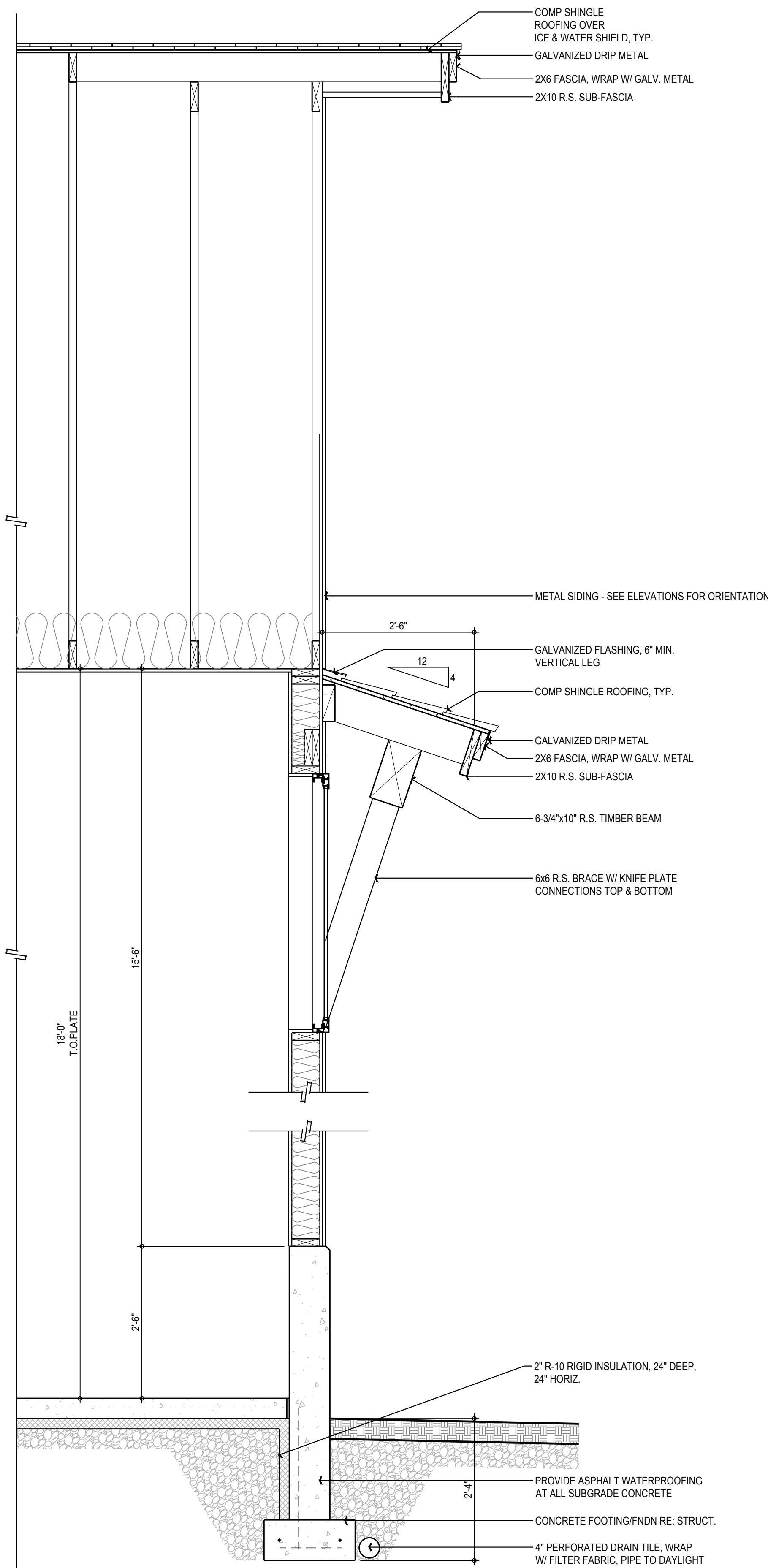
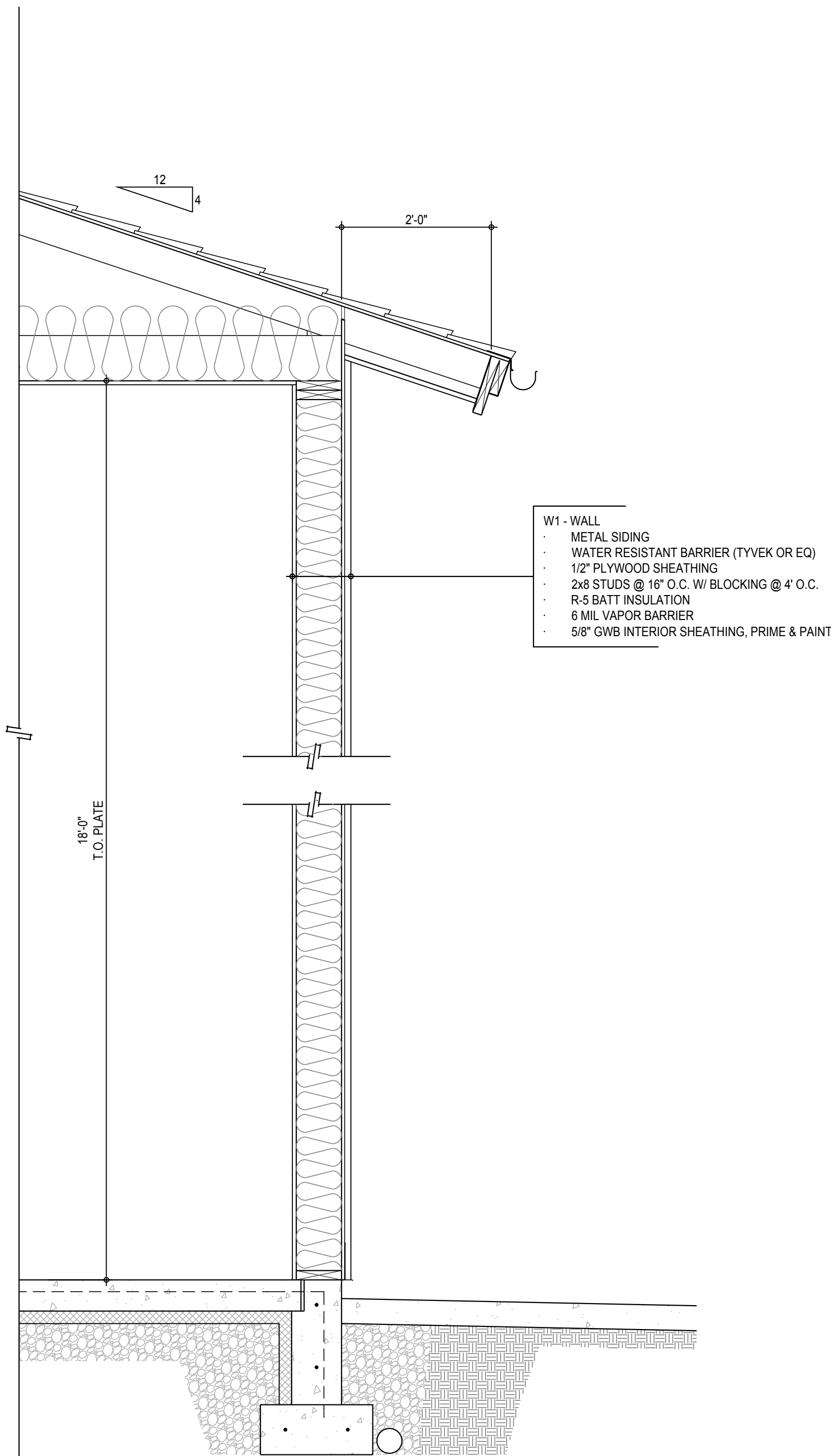
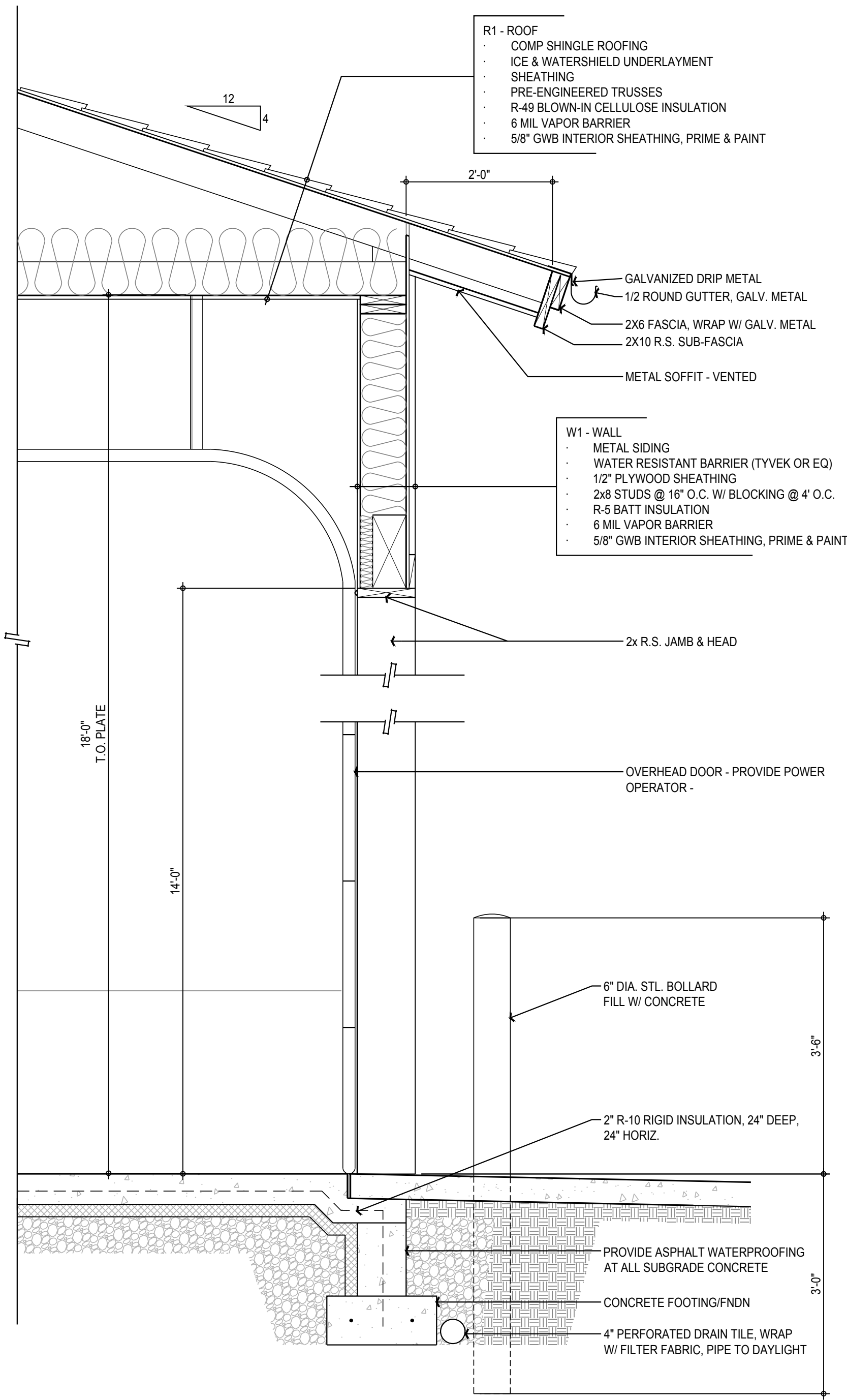
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A302

02-26-23 PERMIT DRAWINGS



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GENERAL CONTRACTOR'S PRIOR REVIEW:

Once the contractor has completed their review of the SSE component drawings, the SER will review the submittal for general conformance with the design of the building and will stamp the submittal accordingly. Review of the Specialty Structural Engineers' (SSE) shop drawings (component design drawings) is for compliance with design criteria and compatibility with the design of the primary structure and does not relieve the SSE of responsibility for that design. All necessary bracing, ties, anchorage, proprietary products shall be furnished and installed per manufacturer's instructions or the SSE's design drawings and calculations. These elements include but are not limited to:

• Prefabricated Wood Roof Trusses

INSPECTIONS, QUALITY ASSURANCE VERIFICATIONS AND TEST REQUIREMENTS

INSPECTIONS: Foundations, footings, under slab systems and framing are subject to inspection by the Building Official in accordance with IBC 110.3. Contractor shall coordinate all required inspections with the Building Official.

SPECIAL INSPECTIONS, VERIFICATIONS AND TESTS: Special Inspections, Verifications and Testing shall be done in accordance with IBC Chapter 17, the STATEMENT AND SCHEDULES OF SPECIAL INSPECTIONS listed in these drawings.

STRUCTURAL OBSERVATION: per IBC Section 1704.6

Structural Observation is the visual observation of the structural system by a registered design professional for general conformance to the approved construction documents. It is not always required on a project, does not include or waive the responsibility for the special inspections and tests required by a Special Inspector per IBC Chapter 17, is not continuous, and does not certify conformance with the approved construction documents.

Structural Observation for this project is not required per IBC Section 1704.6.

CONTRACTOR RESPONSIBILITY: Prior to issuance of the building permit, the Contractor is required to provide the Authority Having Jurisdiction a signed, written acknowledgement of the Contractor's responsibilities associated with the above Statement of Special Inspections addressing the requirements listed in IBC Section 1704.4. Contractor is referred to IBC Sections 1705.12.5 and 1705.12.6 for architectural and MEP building systems that may be subject to additional inspections (based on the building's designated Seismic Design Category listed in the CRITERIA), including anchorage of HVAC ductwork containing hazardous materials, piping systems and mechanical units containing flammable, combustible or highly toxic materials, electrical equipment used for emergency or standby power, exterior wall panels and suspended ceiling systems.

SOILS AND FOUNDATIONS

REFERENCE STANDARDS: Conform to IBC Chapter 18 "Soils and Foundations."

CONTRACTOR'S RESPONSIBILITIES: Contractor shall be responsible to review the Geotechnical Report and shall follow the recommendations specified therein including, but not limited to, subgrade preparations, pile installation procedures, ground water management and steep slope Best Management Practices."

GEOTECHNICAL SUBGRADE INSPECTION: The Geotechnical Engineer shall inspect all sub-grades and prepared soil bearing surfaces, prior to placement of foundation reinforcing steel and concrete. Geotechnical Engineers shall provide a letter to the owner stating that soils are adequate to support the "Allowable Foundation Bearing Pressure(s)" shown below. Assumed values shall be field verified by the Building Official or the Geotechnical Engineer prior to placing concrete.

DESIGN SOIL VALUES:

Safety Factor per Soils Report.....1.5

Allowable Foundation Bearing Pressure.....1500

Passive Lateral Pressure.....250

Coefficient of Sliding Friction.....0.35

PSF – Assumed

PSF/FT - Assumed

Assumed

FOUNDATIONS and FOOTINGS: Foundations shall bear on either on competent native soil or compacted structural fill as per the geotechnical report. Exterior perimeter footings shall bear not less than 24 inches below finish grade, unless otherwise specified by the geotechnical engineer and/or the building official.

FOOTING DEPTH: Tops of footings shall be as shown on plans with vertical changes as indicated with steps in the footings; locations of steps shown as approximate and shall be coordinated with the civil grading plans.

SLABS-ON-GRADE: All slabs-on-grade shall bear on compacted structural fill or competent native soil per the geotechnical report. All moisture sensitive slabs-on-grade or those subject to receive moisture sensitive coatings/covering shall be provided with an appropriate capillary break and vapor barrier/retardant over the subgrade prepared and installed as noted in the geotechnical report, barrier manufacturer's written recommendations and coordinated with the finishes specified by the Architect.

CAST-IN-PLACE CONCRETE

REFERENCE STANDARDS: Conform to:

(1) ACI 301-20 "Specifications for Structural Concrete"

(2) IBC Chapter 19 "Concrete"

(3) ACI 318-19 "Building Code Requirements for Structural Concrete"

(4) ACI 117-10 "Specifications for Tolerances for Concrete Construction and Materials"

FIELD REFERENCE: The contractor shall keep a copy of ACI Field Reference manual, SP-15, "Standard Specifications for Structural Concrete (ACI 301) with Selected ACI and ASTM References."

CONCRETE MIXTURES: Conform to ACI 301 Section 4 "Concrete Mixtures" and IBC Section 1904.1.

MATERIALS: Conform to ACI 301 Section 4.2.1 "Materials" for requirements for cementitious materials, aggregates, mixing water and admixtures.

SUBMITTALS: Provide all submittals required by ACI 301 Section 4.1.2. Submit mix designs for each mix in the table below. Substantiating strength results from past tests shall not be older than 24 months per ACI 318 Section 26.4.3.1 (b).

TABLE OF MIX DESIGN REQUIREMENTS

Member Type/Location	Strength f'c (psi)	Test Age (days)	Nominal Maximum Aggregate	Exposure Class	Max W/C Ratio	Air Content	Notes (1 to 9 Typical UNO)
Foundations	3000	28	1"	-	-	-	9
Exterior Slabs on Grade & Sidewalks	3000	28	1"	-	0.45	6%	-
Interior Slabs on Grade	3000	28	1"	-	0.45	-	-

Table of Mix Design Requirements Notes:

(1) W/C Ratio: Water-cementitious material ratios shall be based on the total weight of cementitious materials. Maximum ratios are controlled by strength noted in the Table of Mix Design Requirements and durability requirements given in ACI 318 Section 19.3.

(2) Cementitious Materials:

a. DCI encourages the reduction of cement content and/or the use of blended hydraulic cements. Where requirements of this section prohibit inclusion of any of these mixes, contact DCI for further coordination.

b. For concrete used in elevated floors, minimum cementitious-materials content shall conform to ACI 301 Table 4.2.1.1.(b) Acceptance of lower cement content is contingent on providing supporting data to the SER for review and acceptance.

c. Cementitious materials shall conform to the relevant ASTM standards listed in ACI 318 Section 26.4.1.1.(a).

(3) Air Content: Conform to ACI 318 Section 19.3.3.1. Minimum standards for exposure class are noted in the table. If freezing and thawing class is not noted, air content given is that required by the SER. Tolerance is ±1-½%. Air content shall be measured at point of placement.

(4) Aggregates shall conform to ASTM C33.

(5) Slump: Conform to ACI 301 Section 4.2.2.1. Slump shall be determined at point of placement.

(6) Chloride Content: Conform to ACI 318 Table 19.3.2.1.

(7) Non-chloride accelerator: Non-chloride accelerating admixture may be used in concrete placed at ambient temperatures below 50°F at the contractor's option.

(8) ACI 318, Section 19.3.1.1 exposure classes shall be assumed to be F0, S0, W0, and C0 unless different exposure classes are listed in the Table of Mix Design Requirements that modify these base requirements.

(9) Structural design is based on strength of 2500 psi and therefore does not require special inspection. The 3000 psi compressive strength is specified for serviceability.

FORMWORK & RESHORING: Conform to ACI 301 Section 2 "Formwork and Form Accessories." Removal of Forms shall conform to Section 2.3.2 except strength indicated in Section 2.3.2.5 shall be 0.75 f'c.

MEASURING, MIXING, AND DELIVERY: Conform to ACI 301 Section 4.3.

HANDLING, PLACING, CONSTRUCTING AND CURING: Conform to ACI 301 Section 5. In addition, hot weather concreting shall conform to ACI 306R-20 and cold weather concreting shall conform to ACI 306R-16.

CONSTRUCTION JOINTS: Conform to ACI 301 Sections. 2.2.2.5 and 5.3.2.6. Construction joints shall be located and detailed as on the construction drawings. Submit alternate locations per ACI 301 Section 5.1.2.3(a) for review and approval by the SER two weeks minimum prior to forming. Use of an acceptable adhesive, surface retardant, portland cement grout or roughening the surface is not required unless specifically noted on the drawings.

EMBEDDED ITEMS: Position and secure in place expansion joint material, anchors and other structural and non-structural embedded items before placing concrete. Contractor shall refer to mechanical, electrical, plumbing and architectural drawings and coordinate other embedded items.

SHRINKAGE: Conventional and post-tensioned concrete slabs will continue to shrink after initial placement and stressing of concrete. Contractor and subcontractor shall coordinate jointing and interior material finishes to provide adequate tolerance for expected structural frame shrinkage and shall include, but not be limited to: curtain wall, dryvit, storefront, skylight, floor finish, and ceiling suppliers. Contact Engineer for expected range of shrinkage.

CONCRETE PLACEMENT TOLERANCE: Conform to ACI 117-10 for concrete placement tolerance.

CONCRETE REINFORCEMENT

REFERENCE STANDARDS: Conform to:

(1) ACI 301-20 "Standard Specifications for Structural Concrete", Section 3 "Reinforcement and Reinforcement Supports."

(2) ACI SP-66(04) "ACI Detailing Manual"

(3) CRSI MSP-09, 28" Edition, "Manual of Standard Practice."

(4) ANSI/AWS D1.4: 2005, "Structural Welding Code - Reinforcing Steel."

(5) IBC Chapter 19-Concrete.

(6) ACI 318-19 "Building Code Requirements for Structural Concrete."

(7) ACI 117-10 "Specifications for Tolerances for Concrete Construction and Materials"

SUBMITTALS: Conform to ACI 301 Section 3.1.2 "Submittals." Submit placing drawings showing fabrication dimensions and placement locations of reinforcement and reinforcement supports.

MATERIALS:

Reinforcing Bars.....ASTM A615, Grade 60, deformed bars.

ASTM A706, Grade 60, deformed bars.

Bar Supports.....CRSI MSP-09, Chapter 3 "Bar Supports."

Tie Wire.....16 gage or heavier, black annealed.

FABRICATION: Conform to ACI 301, Section 3.2.2. "Fabrication", and ACI SP-66 "ACI Detailing Manual."

WELDING: Bars shall not be welded unless authorized. When authorized, conform to ACI 301, Section 3.2.2.2. "Welding", AWS D1.4, and provide ASTM A706, grade 60 reinforcement.

PLACING: Conform to ACI 301, Section 3.3.2 "Placing." Placing tolerances shall conform to ACI 117.

CONCRETE COVER: Conform to the following cover requirements unless noted otherwise in the drawings.

Concrete cast against earth.....3"

Concrete exposed to earth or weather.....2"

Ties in columns and beams.....1-½"

Bars in slabs.....¾"

Bars in walls.....¾"

Exterior bars in Tilt-up Panels.....1"

SPLICES: Conform to ACI 301, Section 3.3.2.7, "Splices". Refer to "Typical Lap Splice and Development Length Schedule" for typical reinforcement splices. Splices indicated on individual sheets shall control over the schedule. Mechanical connections may be used when approved by the SER. For reinforcing within the lateral system (shear walls) and reinforcing connecting the diaphragm slab to the lateral system, mechanical splice strength is increased to develop 125 percent of the specified tensile strength of the splices bar.

FIELD BENDING: Conform to ACI 301 Section 3.3.2.8. "Field Bending or Straightening." Bar sizes #3 through #5 may be field bent cold the first time. Subsequent bends and other bar sizes require preheating. Do not twist bars. Bars shall not be bent past 45 degrees.

TYPICAL CONCRETE REINFORCEMENT: Unless noted on the plans, concrete walls shall have the following minimum reinforcement. Contractor shall confirm minimum reinforcement of walls with SER prior to rebar fabrication.

TABLE of MINIMUM CONCRETE WALL REINFORCING

Wall Thickness	HORIZONTAL Bars	VERTICAL Bars	Location
6"	#4 @ 12" OC	#4 @ 12" OC	center in wall
8"	#5 @ 12" OC	#5 @ 12" OC	center in wall
10"	#4 @ 16" OC EF	#4 @ 16" OC EF	EF = each face
12"	#4 @ 12" OC EF	#4 @ 12" OC EF	EF = each face

WOOD FRAMING

REFERENCE STANDARDS: Conform to:

(1) IBC Chapter 23 "WOOD"

(2) ANSI/AWC NDS - 2018: "National Design Specification (NDS) for Wood Construction - with 2018 NDS Supplement"

(3) ANSI/AWC – SDPWS-2021: Special Design Provisions for Wind and Seismic

(4) APA PDS - 20: "Panel Design Specification"

(5) TPI 1-2014 "National Design Standard for Metal-Plate-Connected Wood Truss Construction"

(6) BCS1 B1 "Guide to Good Practice for Handling, Installing, Restraining & Bracing of Trusses"

(7) DSB-89 "Recommended Design Specification for Temporary Bracing of Metal Plate Connected Wood Trusses"

(8) APA Report TT-045B "Minimum Nail Penetration for Wood Structural Panel Connections Subject to Lateral Loads"

(9) APA Report TT-061C "1-5/16 Inch-Thick I-Joist Flanges and Diaphragm Nail Penetration"

SUBMITTALS: Submit shop drawings to the Architect/Engineer for review. Shop drawings shall include member size, spacing, camber, material type, grade, shop and field assembly details and connections, types and location of bolts and other fasteners. Supply shop drawings for the following:

(1) Glued laminated members

(2) PSL members

(3) LVL members

(4) LSL members

(5) Tapered & Parallel Wood I Joists (Solid web-wood joists)

(6) Panelized wood walls & connection details

(7) Wood Tie-Down Systems

DEFERRED SUBMITTALS: Submit product data and proof of ICC approval for framing members and fasteners that have been designed by others. Submit calculations prepared by the SSE in the state of Idaho for all members and connections designed by others along with shop drawings. All necessary bridging, blocking, blocking panels and web stiffeners shall be detailed and furnished by the supplier. Temporary and permanent bridging shall be installed in conformance with the manufacturer's specifications. Deflection limits shall be as noted under DEFERRED SUBMITTALS section specific details. Products included are:

• Metal plate connected trusses (prefabricated trusses) Conform to IBC Section 2303.4. Truss Supplier to provide design and materials for all permanent truss bracing. Shop drawings shall provide for shapes, bearing points, intersections, hips, and valleys shown on the drawings. The manufacturer shall provide special hip, valley and intersection areas (step down trusses, jack trusses and girder trusses) unless specifically indicated on the plans. Provide all truss-to-truss and truss-to-support connection details and required connection materials. Specify temporary and permanent bracing and connections on the shop drawings. Provide all truss reactions on shop drawings.

IDENTIFICATION: All sawn lumber and pre-manufactured wood products shall be identified by the grade mark or a certificate of inspection issued by the certifying agency.

MATERIALS:

• Sawn Lumber: Conform to grading rules of WWPA, WCLIB or NLGA and Table below. Finger jointed studs acceptable at interior walls only.

TABLE of SOLID SAWN LUMBER

Member Use	Size	Species	Grade
Wall Stud/ Top & Bottom Plates	2x4, 3x4, 2x6, 3x6	Doug Fir Larch	No. 2
Sill Plate (at concrete)	2x4, 3x4, 2x6, 3x6	PT Doug Fir Larch	No. 2
Post	4x4, 4x6, 4x8	Doug Fir Larch	No. 2
Floor or Roof Joist	2x6 through 2x12	Doug Fir Larch	No. 2
Beam	4x8 through 4x12	Doug Fir Larch	No. 2
Beam	6x8 through 6x12	Doug Fir Larch	No. 1
Post or Timber	6x6, 8x8	Doug-Fir Larch	No. 1

• Glued Laminated Timber: Conform to ANSI 117-2020 "Standard Specifications for Structural Glued laminated Timber of Softwood Species, Manufacturing and Design" and ANSI A190.1 "Structural Glued Laminated Timber." Camber all glued laminated beams, except cantilevered and continuous beams, to 300" radius, unless shown otherwise on the plans. Fabricate cantilevered and continuous beams flat, unless shown otherwise on plans.

TABLE of GLULAM AND GRADE

Member	Sizes	Species	Comb. Sym-bol	Uses
Beams	All	DF/DF	24F-V4	Simple Spans

• Wood Structural Sheathing (Plywood): Wood APA-rated structural sheathing includes: all veneer plywood, oriented strand board, waferboard, particleboard, T-11 siding, and composites of veneer and wood based material with T&G joint. Architect may disallow OSB. Conform with Architect. Conform to "Structural Plywood" based on Product Standard PS 1-19 by the U.S. Dept. of Commerce, and "Performance Standard for Wood Structural Panels" based on Product Standard PS 2-18 by the U.S. Dept. of Commerce and "Panel Design Specification" based on APA D510 by the Engineered Wood Association. Unless noted otherwise, sheathing shall comply with the following table:

TABLE of SHEATHING - Use, Minimum Thickness and Minimum APA Rating

Location	Thickness	Span Rating	Plywood Grade	Exposure
Roof	15/32"	32/16	C-D	1
Floor	23/32" T&G	24 OC	STURD-I-FLOOR	1
Walls	15/32"	32/16	C-D	1

Unless noted otherwise on drawings, install roof and floor panels with long dimension across supports and with panel continuous over two or more spans. End joints shall occur over supports.

• Timber Connectors: Shall be "Strong Tie" by Simpson Company as specified in their latest catalog. Alternate connectors by other manufacturers may be substituted provided they have current ICC approval for equivalent or greater load capacities and are reviewed and approved by the SER prior to ordering. Connectors shall be installed per the manufacturer's instructions. Where connector straps connect two members, place one-half of the nails or bolts in each member. Where straps are used as hold-downs, nail straps to wood framing just prior to drywall application, as late as possible in the framing process to allow the wood to shrink and the building to settle. Premature nailing of the strap may lead to strap buckling and potential finish damage.

Where connectors are in exposed exterior applications in contact with preservative treated wood (PT) other than CCA, connectors shall be either batch hot-dipped galvanized (HDG), mechanically galvanized (ASTM B695, Class 55 minimum) stainless steel, or provided with 1.85 oz/sf of zinc galvanizing equal to or better than Simpson ZMAX finish.

Fasteners (nails, bolts, screws, etc) attaching timber connectors (joist hangers, post caps and bases, etc) to PT wood shall have similar corrosion resistance properties (matching protective treatments) as the protected connector. Fasteners (nails, bolts, screws, etc) attaching sawn timber members or sheathing (shear walls) to PT wood shall be corrosion resistant; nails and lag bolts shall be either HDG (ASTM A153) or stainless steel. Verify the suitability of the fastener protection/coating with the wood treatment chemical manufacturer/ supplier.

Provide washers under the heads and nuts of all bolts and lag screws bearing on wood.

• Lag Bolts/Bolts: Conform to ASTM A307 and IBC Section 2304.10.

• Nails and Staples: Conform to ASTM F1667 and IBC Sections 2303.6 and 2304.10.

NAILING REQUIREMENTS: Conform to IBC Section 2304.10 "Connectors and fasteners." Unless noted on plans, nail per Table 2304.10.2. Nailing for roof/floor diaphragms/shear walls shall be per drawings. Nails shall be driven flush and shall not fracture the surface of sheathing. Alternate nails may be used but are subject to review and approval by the Structural Engineer. Substitution of staples for the nailing of rated sheathing is subject to review by the structural engineer prior to construction.

STANDARD LIGHT-FRAME CONSTRUCTION: Unless noted on the plans, construction shall conform to IBC Section 2308 "Conventional Light-Frame Construction."

NAILERS ON STEEL COLUMNS and BEAMS: Wood 3x nailers are generally required on all HSS columns and steel beams abutting or embedded within wood framing. Unless noted otherwise, attach with 5/8" diameter bolts or welded studs at 16" on centers. Unless noted otherwise, wood nailers on beams supporting joist hangers shall not overhang the beam flange by more than ¼".

WOOD SHRINKAGE AND EXPANSION: Wood materials will expand or contract based on relative changes in moisture. The contractor is responsible for means and methods of construction related to mitigating and managing the effects of changes in moisture.

MOISTURE CONTENT: The contractor shall make provisions during handling and construction to prevent the structural wood members from exceeding the appropriate moisture content limits. The moisture content for solid sawn wood material used for this project shall not exceed 19%. The moisture content for engineered wood products, laminated lumber and sheathing shall not exceed the limits required by the manufacturer or 12%, whichever is less. The moisture content limits may be more stringent for particular product requirements (eg. finishes, cladding, insulation systems, etc.). The contractor shall refer to the Architect's drawings, project specifications, or installer/product requirements for additional requirements.

SHRINKAGE COMPENSATION FOR MECHANICAL, ELECTRICAL, AND PLUMBING SYSTEMS: MEP systems, including ductwork, pipes, and other elements that run continuously between levels shall be installed/designed in such a manner to accommodate shrinkage in the wood framing. Wood shrinkage amounts will vary depending on the construction process and materials used. The anticipated shrinkage under typical conditions is expected to range between 1/8" and 1/4" per floor.

CLADDING COMPATIBILITY: The Architect/Owner and contractor shall review the cladding, finishes, insulation systems, other non-structural components and construction procedures proposed for the project with respect to their performance over wood framing. EIFS systems should be avoided on wood-framed projects due to problems with moisture proofing. Note that DCI is not responsible for the attachment of the cladding to the wood studs which needs to be verified and provided by the cladding supplier.

PRESERVATIVE TREATMENT (PT): Wood materials that are required to be "treated wood" in accordance with IBC Section 2304.12. "Protection Against Decay and Termites" shall conform to the appropriate standards of the American Wood Protection Association (AWPA) for sawn lumber, glued laminated timber, round poles, wood piles and marine piles. Follow American Lumber Standards Committee (ALSC) quality assurance procedures. Products shall bear the appropriate mark. Fasteners or anchors in treated wood shall be of stainless steel or hot-dipped galvanized or as per IBC 2304.10.6.

Mud sill plates in normally dry interior applications may be treated with Sodium Borate (DOT - Disodium Octabrate Tetrahydrate) as recent studies have noted less connector corrosion potential than other available wood treatments or the original CCA treated sill plates. Wood treated with Sodium Borate shall be protected during shipment, storage and installation to minimize leaching of the water-soluble preservative from the lumber. Sodium borate pressure treated plates do not require hot-dipped galvanized connectors.

If using preservative treatments other than CCA or sodium borate, fasteners must be hot dipped galvanized or stainless steel. Wood treated with Alkaline Copper Quaternary (ACQ) requires steel components in contact with the wood to be stainless (nails, bolts, screws, washers & lag screws). Fasteners (nails, bolts, screws, washers & lag screws) attaching timber connectors (joist hangers, post caps and bases, etc) to PT wood shall have similar corrosion resistance properties (matching protective treatments) as the protected connector; that is, use hot dipped galvanized or stainless-steel fasteners. Fasteners (nails, bolts, screws, washers & lag screws) attaching sawn timber members or sheathing (shear walls) to Pressure Treated wood shall be corrosion resistant (hot dipped galvanized or stainless steel).

Always verify the suitability of the fastener protection/coating with the wood treatment chemical manufacturer/ supplier.

Fire Retardant Treated (FRT) Wood: Wood material that is required to be Fire Retardant Treated Wood to conform to IBC section 2303.2 – "Fire-retardant-treated wood." Submit ICC report to SEOR for review and approval prior to construction.

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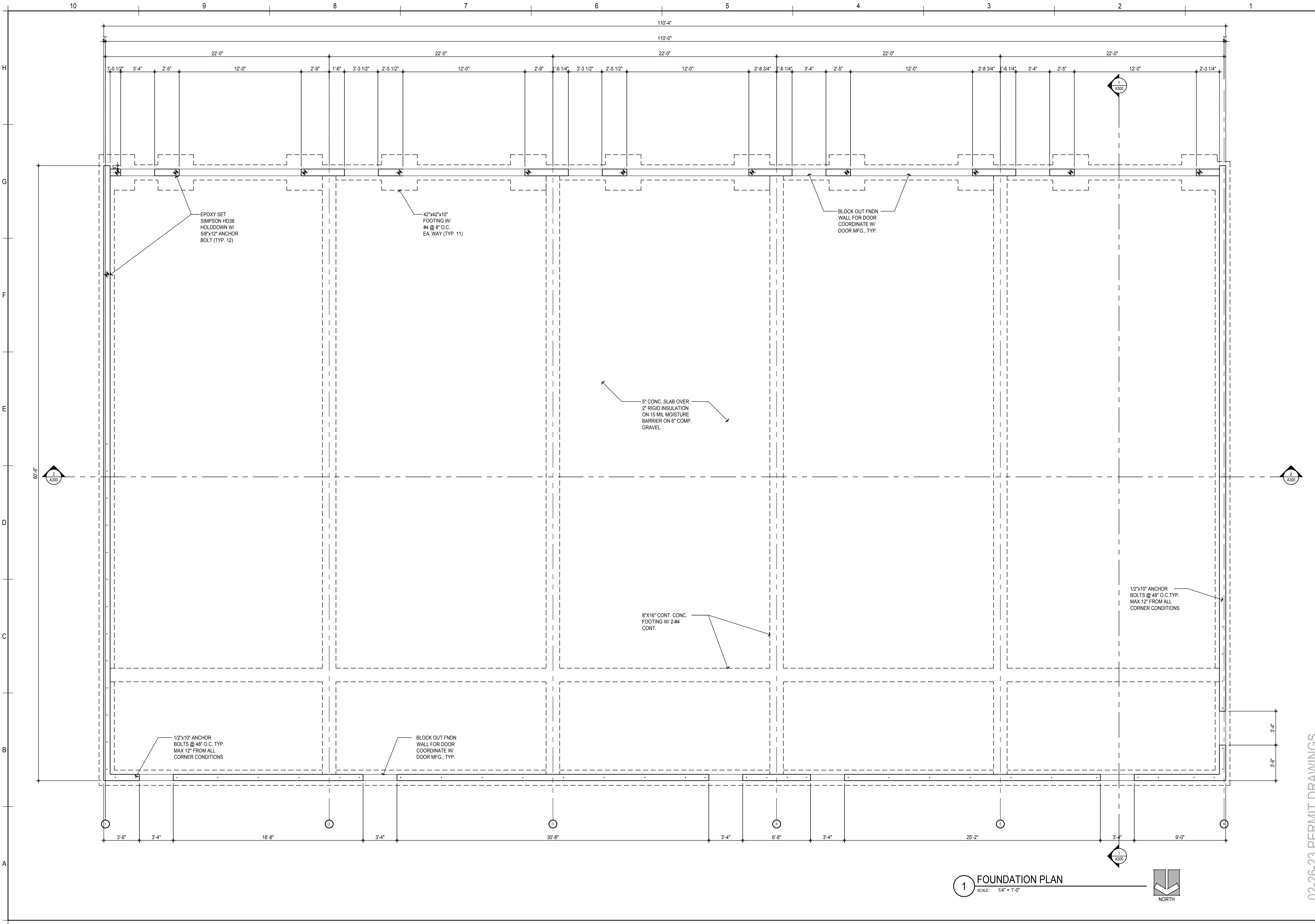
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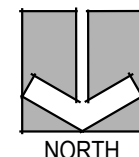
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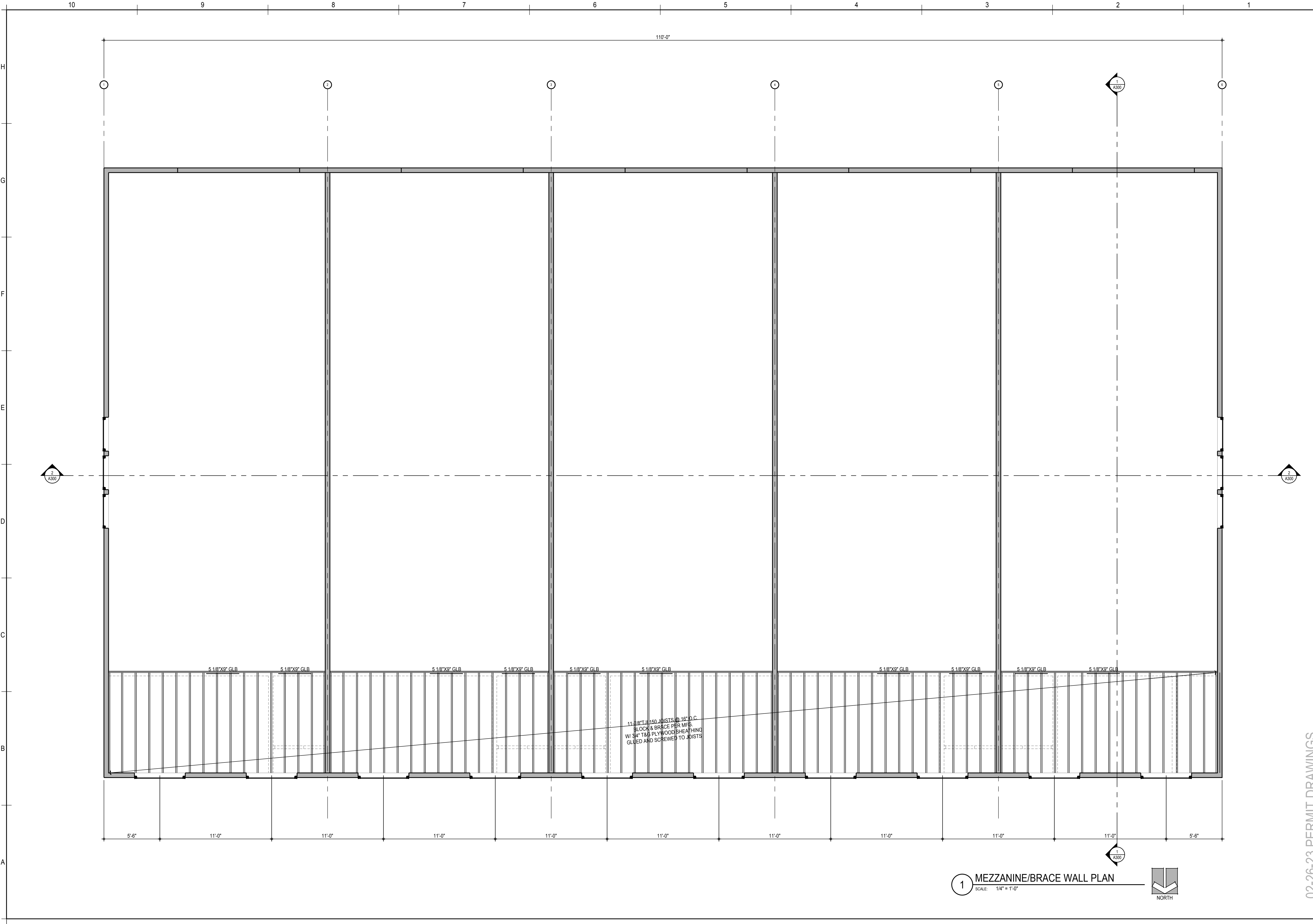
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1 FOUNDATION PLAN
SCALE: 1/4" = 1'-0"



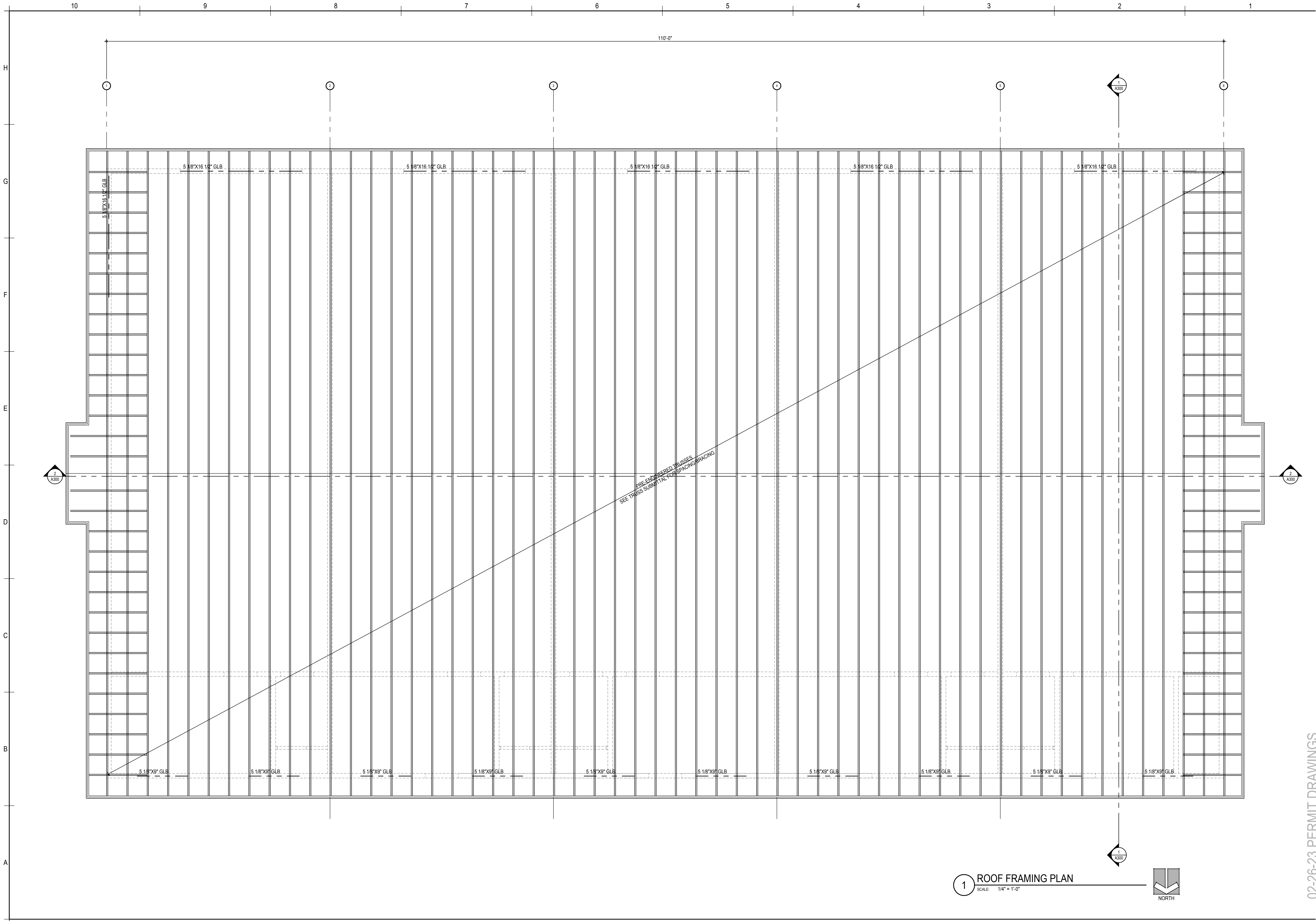


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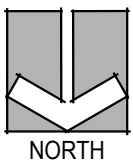
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1 ROOF FRAMING PLAN
SCALE: 1/4" = 1'-0"




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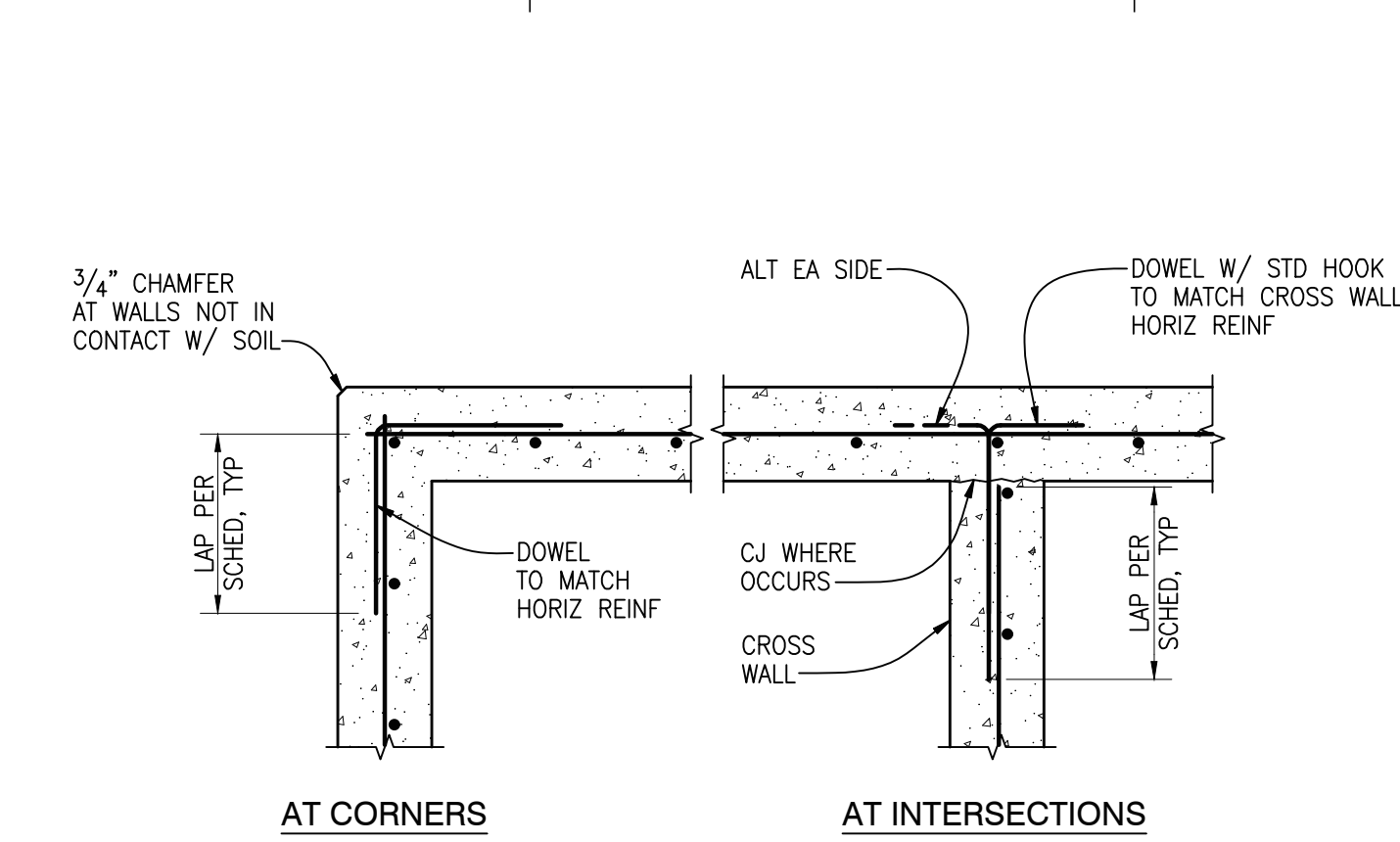
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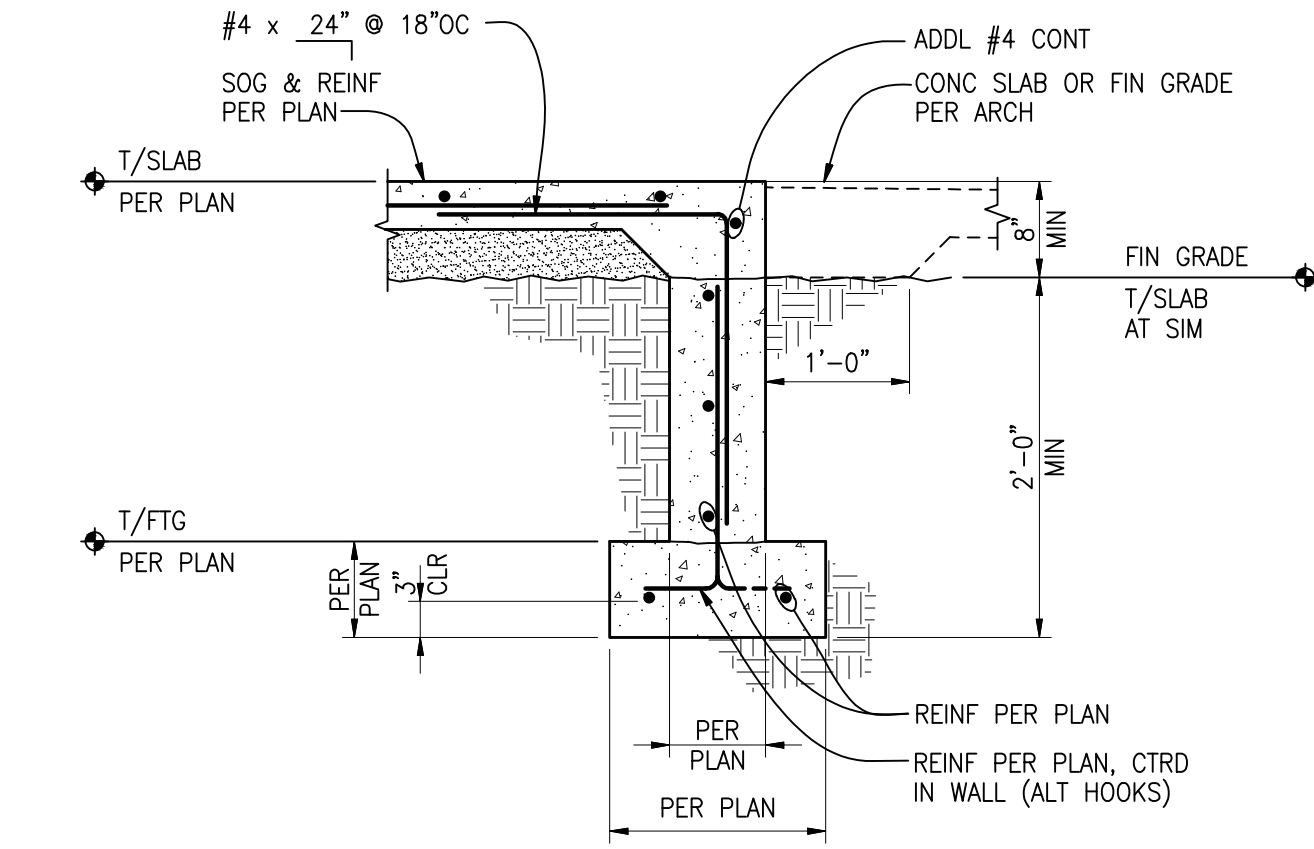
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- NOTES:**
1. SPLICE LENGTHS PER LAP SPLICE AND DEVELOPMENT LENGTH SCHEDULE.
 2. WALL REINFORCING PER PLAN OR ELEVATIONS, SECTIONS AND DETAILS.
 3. AT FOOTINGS AND STEMWALLS, CORNER REINFORCING TO MATCH FOOTING AND STEMWALL HORIZONTAL REINFORCING.

NOTES:		#14	93	N/A	121	N/A	38
1. ALL TABULATED VALUES ARE IN INCHES.		#18 <td>124<td>N/A<td>161<td>N/A<td>50</td></td></td></td></td>	124 <td>N/A<td>161<td>N/A<td>50</td></td></td></td>	N/A <td>161<td>N/A<td>50</td></td></td>	161 <td>N/A<td>50</td></td>	N/A <td>50</td>	50
2. VALUES FOR UNCAOTED REINFORCING AND NORMAL WEIGHT CONCRETE WITH CLEAR SPACING > db, CLEAR COVER > db AND MINIMUM STIRRUPS OR TIES THROUGHOUT Ld OR CLEAR SPACING > 2db AND CLEAR COVER > db.							
3. DEVELOP ALL REINFORCING IN STRUCTURAL SLABS WITH MINIMUM DEVELOPMENT LENGTH Ld.							
4. Ldh = DEVELOPMENT LENGTH OF BAR WITH STANDARD HOOK.							
5. TOP BAR = HORIZONTAL BAR WITH MORE THAN 12" OF FRESH CONCRETE BELOW OR AS NOTED ON DOCUMENTS AS "TOP BAR".							
6. LAP SPICE OF DIFFERENT SIZE BARS TO BE THE LARGER OF Ld OF THE LARGER BAR OR SPlice LENGTH OF THE SMALLER BAR.							
7. LAP SPICE #14 AND #18 BARS IS NOT PERMITTED. LAP SPICE OF SMALLER BARS TO #14 AND #18 BARS IS NOT PERMITTED.							
8. LAP SPICE OF DIFFERENT GRADES OF REINFORCING TO BE THE LARGER OF Ld OF THE HIGHER GRADE BAR OR SPlice LENGTH OF THE LOWER GRADE BAR.							

03402 (SINGLE MAT) SCALE: $\frac{3}{4}" = 1'-0"$



$D = 6d$ FOR #3 THRU #8
 $D = 8d$ FOR #9, #10 & #11
 $D = 10d$ FOR #14 & #18

ALL REINFORCING EXCEPT COLUMN TIES AND BEAM STIRRUPS

STIRRUP

STIRRUP OR TIE

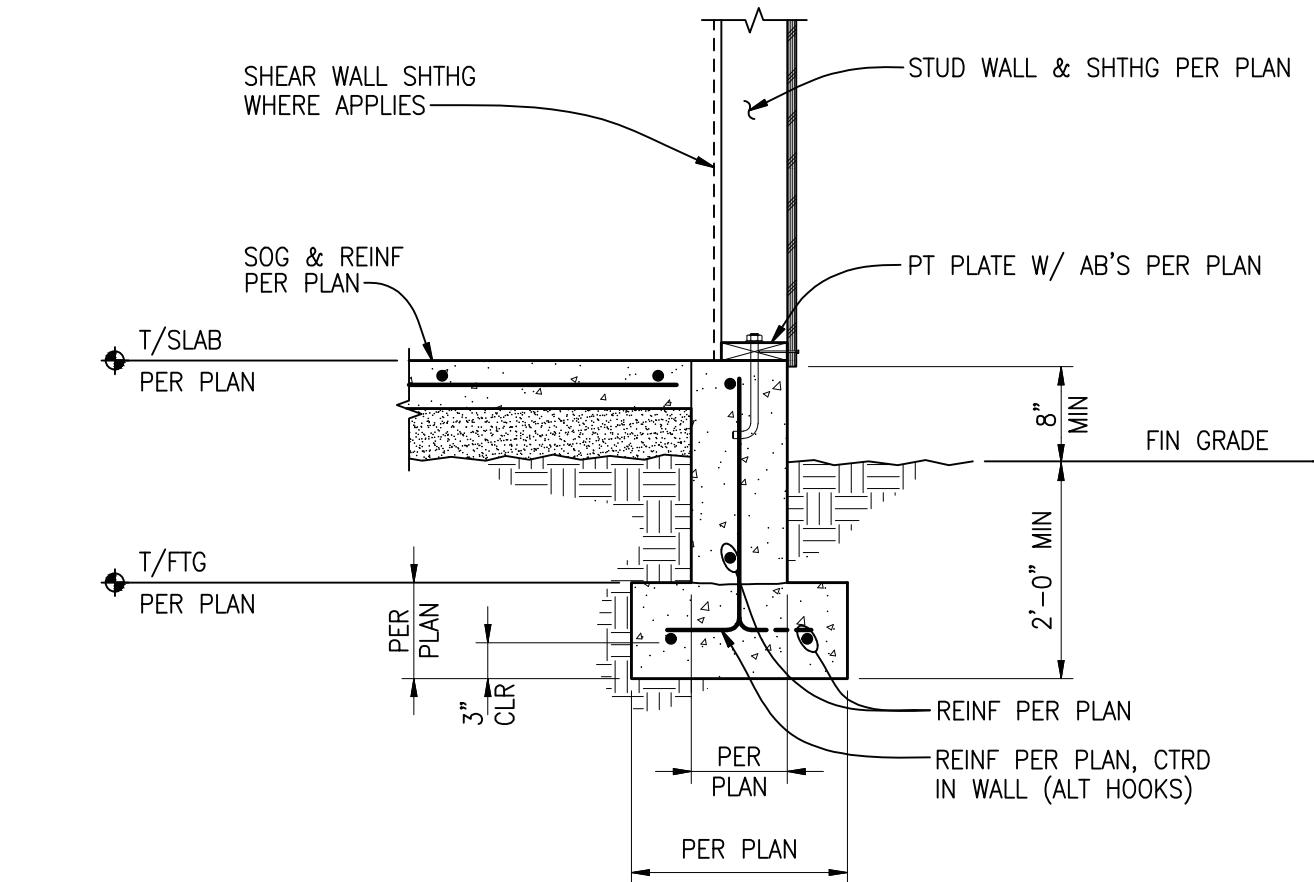
$D = 4d$ FOR #3, #4 & #5
 $D = 6d$ FOR #6, #7 & #8

BEAM OR COLUMN CROSSTIES

BEAM STIRRUPS AND COLUMN TIES

$d = \text{BAR DIAMETER, } D = \text{BEND DIAMETER}$

03033M SCALE: $\frac{3}{4}" = 1'-0"$



NOTE: TIES AND CROSSIES FOR SHEAR WALL BOUNDARY ELEMENTS SHALL BE DETAILED AS COLUMN TIES/CROSSIES.

STANDARD HOOKS AND BENDS

03400 (FOR REVISIONS TO STANDARD HOOKS & BENDS REF TO CURRENT ACI) SCALE: NONE

5/8"ø x 24" SMOOTH BAR
@ 24"OC, GREASE ONE SIDE
IN DOWEL INSERT

T/SLAB
PER PLAN

REINF PER PLAN

PER PLAN

SUBGRADE PREPARATION
PER GEOTECH REPORT

VAPOR BARRIER
WHERE REQD

COMPACTED STRUCT FILL
OR COMPETENT NATIVE SOIL
PER GEOTECH REPORT

CONSTRUCTION JOINT

REINF PER PLAN

T/SLAB
PER PLAN

PER PLAN

SUBGRADE PREPARATION
PER GEOTECH REPORT

VAPOR BARRIER
WHERE REQD

COMPACTED STRUCT FILL
OR COMPETENT NATIVE SOIL
PER GEOTECH REPORT

1/4" SAWCUT DEPTH OR
1/4" PREMOULDED JOINT (1 1/2" MIN)

CONTROL JOINT

NOTES:

1. CONSTRUCTION JOINT IS A JOINT BETWEEN DIFFERENT POURS. CONTROL JOINT IS A CRACK CONTROL JOINT WITHIN THE SAME POUR.
2. USE "EARLY ENTRY DRY-CUT SAW" AS SOON AS POSSIBLE WITHOUT CAUSING RAVELING OF CONCRETE EDGES. SAWCUT ALONG SHORT DIRECTION OF POUR FIRST.
3. ALIGN A CONSTRUCTION OR CONTROL JOINT WITH RE-ENTRANT SLAB CORNERS, EACH WAY, TYPICAL.
4. PROVIDE CONSTRUCTION/CONTROL JOINT TO ENCLOSE APPROXIMATE SQUARE AREAS 225 SQUARE FEET MAXIMUM, WITH A MAXIMUM PANEL ASPECT RATIO OF 1.3 TO 1.0.
5. CONTRACTOR TO SUBMIT CONSTRUCTION/CONTROL JOINT PLAN TO STRUCTURAL ENGINEER OF RECORD FOR REVIEW/APPROVAL.

TYPICAL SLAB ON GRADE

03031M SCALE: $\frac{3}{4}" = 1'-0"$

TYPICAL SLAB ON GRADE
JOINT DETAILS WITH REINFORCING

03201

SCALE: $\frac{3}{4}"=1'-0"$

12

PONDERAY COMMERCIAL STORAGE
BONNER MALL WAY
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S201

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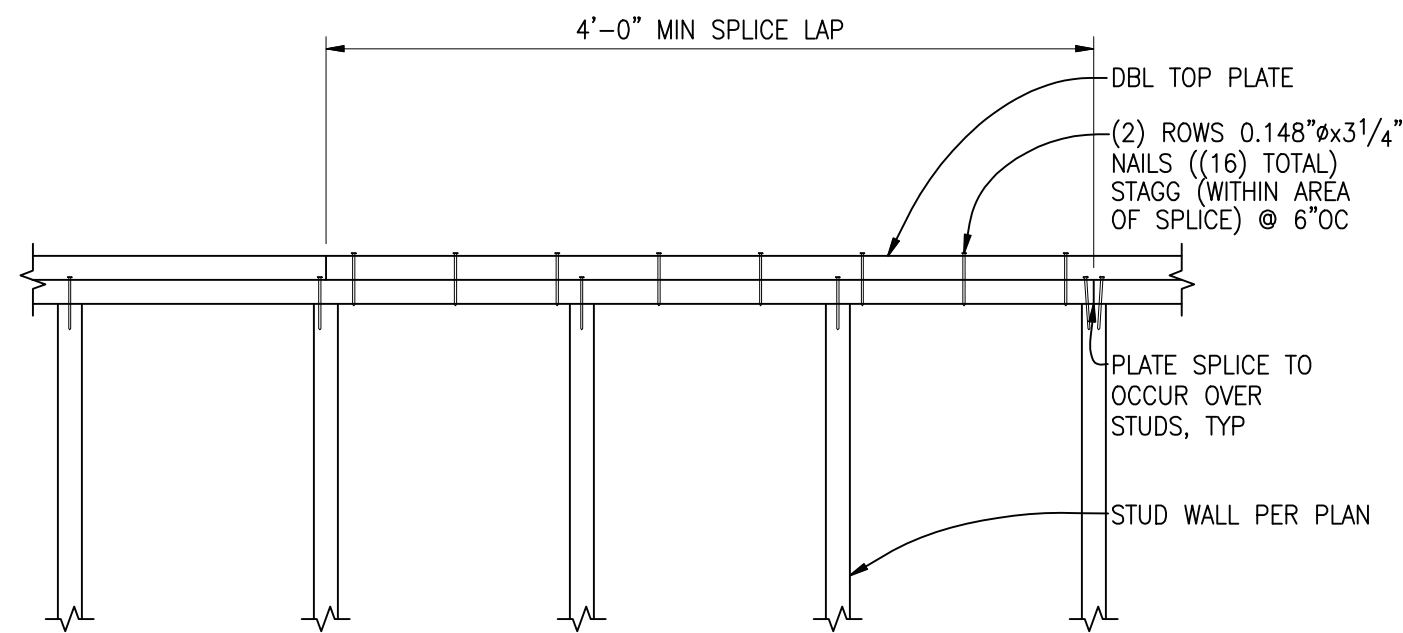
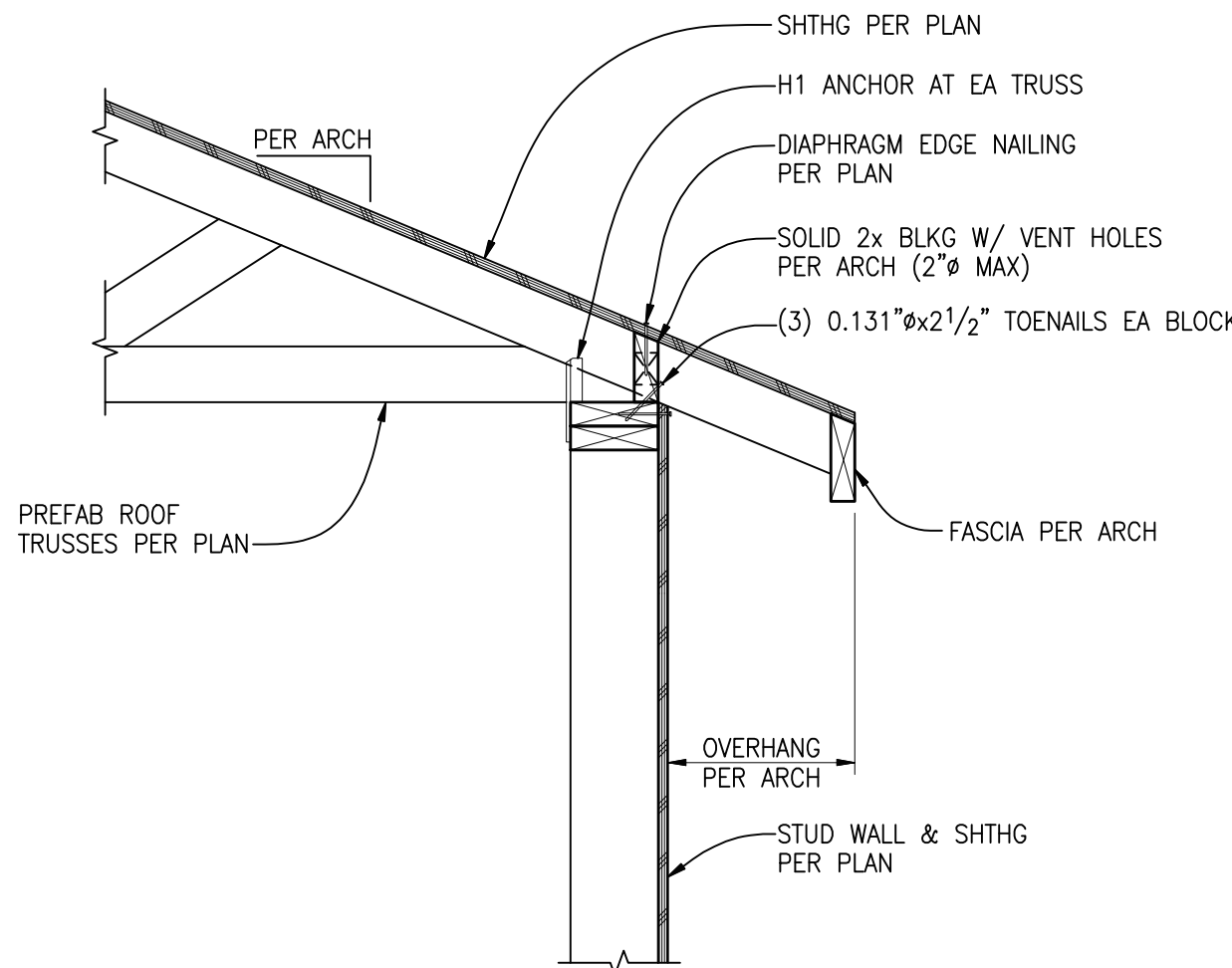
B

A

HOLD-DOWN/STRAP SCHEDULE - DOUG-FIR STUDS									
[1, 2, 7, 11] ← INDICATES FOOTNOTES									
	TYPE	NUMBER OF STUDS/POST [3, 12]	NAILS, SCREWS OR BOLTS	DIAMETER [10]	ANCHOR [4]				NOTES
					CONCRETE EMBEDMENT/CAPACITY				
					STEMWALL [5]		FOOTING		
					EMBED CIP [6, 14]	CAPACITY	EMBED CIP [6]	CAPACITY	
CONCRETE TO WOOD	HDU2	(2) 2x	(6) SDS 1/4x2 1/2	5/8"Ø	10"	3.1k	8"	3.1k	----

NOTES:

- [1] SOME HOLD-DOWN TYPES NOTED MAY NOT BE USED ON THIS PROJECT.
- [2] TYPICAL HOLD-DOWN DETAILS PER 2/S3.1. ANCHOR REINFORCEMENT REQUIRED AT STEMWALLS.
- [3] PROVIDE PANEL EDGE NAILING PER SHEAR WALL SCHEDULE AT HOLD-DOWN STUDS/POST.
- [4] BASED ON MINIMUM f'c = 3000 PSI CONCRETE.
- [5] STEMWALLS SHALL BE 8" WIDE x 18" TALL MINIMUM.
- [6] CAST-IN-PLACE (CIP) TYPE THREADED RODS AT HOLD-DOWNS SHALL HAVE TWO HEX HEAD NUTS WITH OVERSIZED WASHER.
- [7] INCLUDES 1.6 LOAD DURATION INCREASE FOR WOOD.
- [8] BASED ON 11 7/8" DEEP FLOOR JOIST.
- [9] TOTAL NAILS SPECIFIED, USE HALF THE NAILS AT THE STUDS ABOVE AND BELOW LEVEL BEING CONNECTED.
- [10] AT PRESSURE TREATED SILLS, USE HOT DIPPED GALVANIZED BOLTS.
- [11] POST INSTALLED HOLD-DOWN OPTIONS MAY BE AVAILABLE AT SOME CONDITIONS. CONTACT ENGINEER OF RECORD PRIOR TO CONSTRUCTION.
- [12] NAIL LAMINATE MULTIPLE 2x STUDS WITH PLATE NAILING PER SHEAR WALL SCHEDULE.
- [13] MIDWALL/CORNER WALL END
- [14] STUD WALLS SHALL BE 2x6, CENTER HOLD-DOWN IN STUD WALL.



NOTE:

FLOOR/ROOF JOISTS NOT SHOWN FOR CLARITY.

HOLD-DOWN/STRAP SCHEDULE - DOUG FIR STUDS

01420M

SCALE: NONE

2

EXTERIOR WALL PERPENDICULAR TO ROOF TRUSSES

06065AM

SCALE: 1"=1'-0"

3

TYPICAL PLATE SPLICE DETAIL

06904

SCALE: 1"=1'-0"

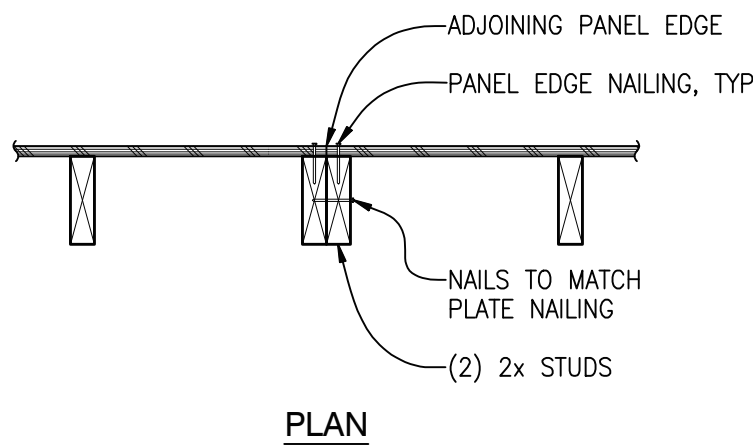
4

SHEAR WALL SCHEDULE [W6] > FOR 0.131"Øx2 1/2" NAILS IN DOUG-FIR LARCH (2018 IBC) [17]							
SOME SHEAR WALL TYPES NOTED MAY NOT BE USED ON THIS PROJECT.							
WALL TYPE	WALL SHEATHING APA-RATED [1, 2, 12, 13]	NAIL SIZE & SPACING AT ALL PANEL EDGES [4, 5]	BLOCKING & STUD SIZE AT ADJOINING PANEL EDGES [3, 6, 14]	RIM JOIST OR BLOCKING CONN TO TOP PLATE BELOW [7, 8]	2x PLATE ATTACHMENT NAILING TO WOOD RIM JOIST OR BLOCKING BELOW	SILL PLATE ATTACHMENT ANCHOR BOLT TO CONCRETE BELOW [10]	SHEAR CAPACITY LBS/FT
[W6] >	1 5/32"	0.131"Øx2 1/2" @ 6"OC	2x	CLIP @ 16"OC	0.148"Øx3 1/4" @ 8"OC	5/8"Ø @ 48"OC	260

NOTES:

- [1] INSTALL PANELS EITHER HORIZONTALLY OR VERTICALLY.
- [2] WHERE SHEATHING IS APPLIED ON BOTH SIDES OF WALL, PANEL EDGE JOINTS ON 2x FRAMING SHALL BE STAGGERED SO THAT JOINTS ON THE OPPOSITE SIDES ARE NOT LOCATED ON THE SAME STUDS.
- [3] BLOCKING IS REQUIRED AT ALL PANEL EDGES.
- [4] PROVIDE SHEAR WALL SHEATHING AND NAILING FOR THE ENTIRE LENGTH OF THE WALLS INDICATED ON THE PLANS. ENDS OF FULL HEIGHT WALLS ARE DESIGNATED BY WINDOWS, OR DOORWAYS OR AS DESIGNATED ON PLANS. HOLD-DOWN REQUIREMENTS PER PLANS. (ALTERNATE NOTE: WALLS SHOWN WITH HORIZONTAL STRAPS BELOW AND/OR ABOVE OPENINGS REQUIRE SHEATHING, SHEAR WALL NAILING, ETC. ABOVE AND BELOW ALL OPENINGS).
- [5] SHEATHING EDGE NAILING IS REQUIRED AT ALL HOLD-DOWN POSTS. EDGE NAILING MAY ALSO BE REQUIRED TO EACH STUD USED IN BUILT-UP HOLD-DOWN POSTS. ADDITIONAL INFORMATION PER HOLD-DOWN DETAILS.
- [6] INTERMEDIATE FRAMING TO BE 2x MINIMUM MEMBERS. ATTACH SHEATHING TO INTERMEDIATE FRAMING WITH 0.131"Øx2 1/2" NAILS AT 12"OC WHERE STUDS ARE SPACED AT 16"OC AND 0.131"Øx2 1/2" NAILS AT 6"OC WHERE STUDS ARE SPACED AT 24"OC.
- [7] BASED ON 0.131"Øx1 1/2" NAILS USED TO ATTACH FRAMING CLIPS DIRECTLY TO FRAMING. USE 0.131"Øx2 1/2" NAILS WHERE INSTALLED OVER SHEATHING.
- [8] FRAMING CLIPS: A35 OR LTP5 OR APPROVED EQUIVALENT.
- [9] WHERE BOTTOM PLATE ATTACHMENT SPECIFIES (2) ROWS OF NAILS OR SCREWS, PROVIDE DOUBLE JOIST, RIM JOIST OR EQUAL BELOW. STAGGER NAILS/SCREWS IN ROWS 1 1/2" APART MINIMUM.

- [10] ANCHOR BOLTS SHALL BE PROVIDED WITH HOT DIPPED GALVANIZED STEEL PLATE WASHERS 0.229"x3"x3" MIN. THE HOLE IN THE PLATE WASHER MAY BE DIAGONALLY SLOTTED 13/16"x13/4" PROVIDED A STANDARD CUT WASHER IS PLACED BETWEEN THE PLATE WASHER AND NUT. PLATE WASHER TO EXTEND TO WITHIN 1/2" OF THE EDGE OF THE SILL PLATE ON THE SIDE(S) WITH SHEATHING. AT 2x6 WALLS WITH SHEATHING ON BOTH SIDES USE PLATE WASHER 0.229"x4 1/2"x4 1/2" MINIMUM. EMBED ANCHOR BOLTS 7" MINIMUM INTO THE CONCRETE.
- [11] PRESSURE TREATED MATERIAL CAN CAUSE EXCESSIVE CORROSION IN THE FASTENERS. PROVIDE HOT-DIPPED GALVANIZED (ELECTRO-PLATING IS NOT ACCEPTABLE) NAILS AND CONNECTOR PLATES (FRAMING ANGLES, ETC.) FOR ALL CONNECTORS IN CONTACT WITH PRESSURE TREATED FRAMING MEMBERS. ADDITIONAL INFORMATION PER STRUCTURAL NOTES.
- [12] 7/16" APA-RATED SHEATHING (OSB) MAY BE USED IN PLACE OF 1 5/32" SHEATHING PROVIDED THAT ALL STUDS ARE SPACED AT 16"OC MAXIMUM.
- [13] WHERE WOOD SHEATHING (W) IS APPLIED OVER GYPSUM SHEATHING (G), CONTACT THE ENGINEER OF RECORD FOR ALTERNATE NAILING REQUIREMENTS.
- [14] AT ADJOINING PANEL EDGES, (2) 2x STUDS NAILED TOGETHER MAY BE USED IN PLACE OF SINGLE 3x STUD. DOUBLE 2x STUDS SHALL BE CONNECTED TOGETHER BY NAILING THE STUDS TOGETHER WITH 3" LONG NAILS OF THE SAME SPACING AND DIAMETER AS THE PLATE NAILING, PER SECTION.
- [15] CONTACT THE STRUCTURAL ENGINEER OF RECORD FOR ADHESIVE OR EXPANSION BOLT ALTERNATIVES TO CAST-IN-PLACE ANCHOR BOLTS. SPECIAL INSPECTION MAY BE REQUIRED.
- [16] NAIL STUDS TO 3x SILL PLATES WITH EITHER (2) 0.148"Øx4" END NAILS OR (4) 0.131"Øx2 1/2" TOENAILS.
- [17] [WX] > WHERE "W" INDICATES WOOD SHEATHING AND "X" INDICATES EDGE NAIL SPACING.
- [18] EDGE NAILS SHALL BE LOCATED 3/8" FROM PANEL EDGES.



SHEAR WALL SCHEDULE - DOUG-FIR LARCH

01430M

SCALE: NONE

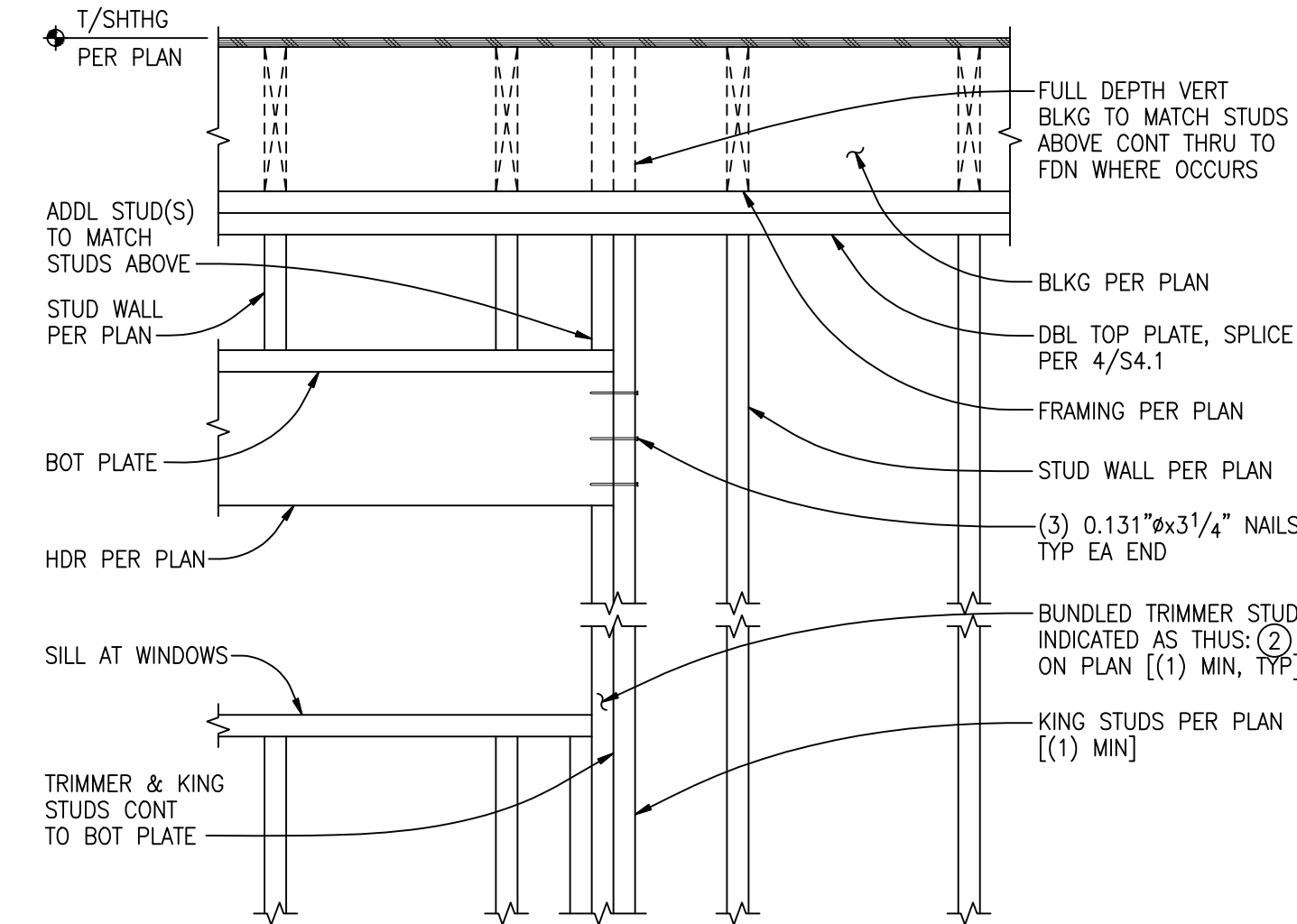
11

EXTERIOR WALL PARALLEL TO ROOF TRUSSES

06064M (GABLE END)

SCALE: 1"=1'-0"

12

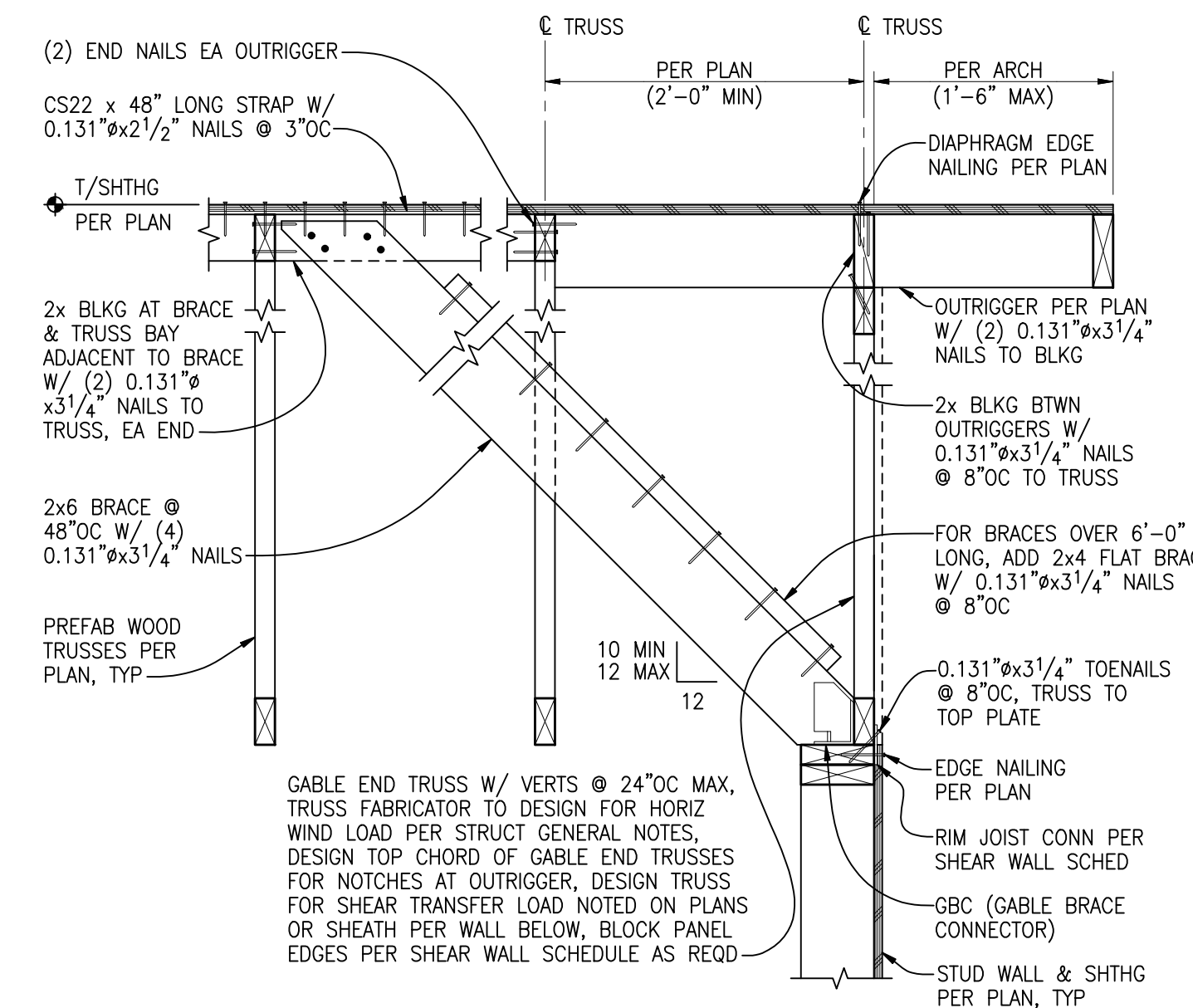


TYPICAL HEADER

06210M

SCALE: 1"=1'-0"

8



926 Peachtree Dr.
Moscow, ID 83843
208.310.0289

PONDERAY COMMERCIAL STORAGE

BONNER MALL WAY
PONDERAY, ID 83852

PCS BLDG 1	
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